

Exhibit E



**HEATH & ASSOCIATES**  
Transportation Planning & Engineering

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## TRAFFIC IMPACT ANALYSIS

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### Whittier Elementary School

Fircrest, Washington

October 15, 2025

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# WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

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## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

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## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

### 1. INTRODUCTION

Heath & Associates has been retained to prepare a Traffic Impact Analysis (TIA) for the proposed reconstruction of Whittier Elementary School within the City of Fircrest. The study evaluates existing traffic conditions in the vicinity of the site and compares them to future conditions with and without the proposed project. The analysis considers traffic operations, safety, and circulation, and concludes with findings and recommended mitigation measures, if warranted, to ensure the school's redevelopment can be accommodated within the surrounding transportation network.

### 2. PROJECT DESCRIPTION

Whittier Elementary School, located at 777 Elm Tree Lane, currently serves approximately 320 students (Pre-K through 3rd grade) with a staff of about 40. The proposal would replace the existing campus with a new school building, increasing capacity to up to 380 students and 45 staff. As part of site development, the three existing buildings (totaling 52,383 sq ft) would be demolished prior to construction. The project will include 50 on-site parking stalls, with a bus drop-off and pick-up area created on the north side of the site. Buses currently stage on Elm Tree Lane. Today, parents and guardians drop-off and pick-up students along Annapolis Avenue, traveling east and then north from Alameda Avenue. With the new project, traffic flow will be reversed. Parents will enter the site from Elm Tree Lane and exit onto Alameda Avenue.

The 4.96-acre site (Tax Parcel #0220114002) is bounded by Elm Tree Lane to the north, Alameda Avenue to the west, and Annapolis Street to the south and east. Today, vehicular access is provided via driveways on Alameda Avenue and Elm Tree Lane, with student drop-off and pick-up occurring along Annapolis Street. Although Annapolis Street accommodates two-way traffic, most vehicles approach from the south and west and then exit to Elm Tree Lane.

**Figure 1** displays the vicinity map with the subject site highlighted in blue. A conceptual site plan is provided in **Figure 2**.



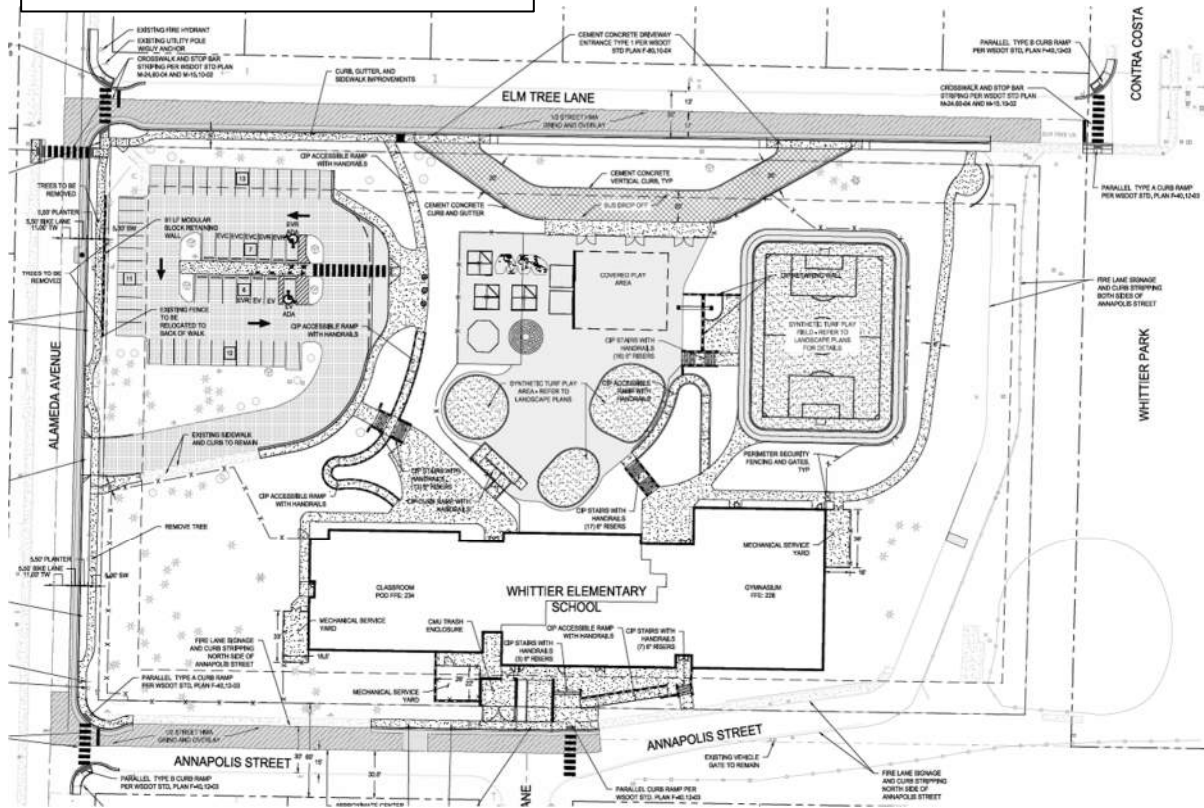
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**Figure 1: Vicinity Map**



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**Figure 2: Conceptual Site Plan**



As illustrated above, access to the parking lot would continue from Alameda Avenue. A new bus loop is proposed from Elm Tree Lane, and the existing drop-off and pick-up lane on Annapolis Street, currently one-way northbound, would be reoriented to one-way southbound.



## 3. EXISTING CONDITIONS

### 3.1 Existing Street System

**Table 1** summarizes information on the roadways around Whittier Elementary School.

**Table 1: Roadway Network**

Roadway Classification	Roadway	Speed Limit	Lanes	Street Parking	Sidewalk	Bike Facilities
Major Collector	Alameda Ave	25	2	No	Yes <sup>1</sup>	No
Local Roads	Elm Tree Lane	25	2	Yes	South side	No
	Annapolis St	25	2	No	North side	No
	Contra Costa Ave	25	2	Yes	West side	No

<sup>1</sup> The sidewalk on the east side of Alameda ends south of Rosewood Lane.

### 3.2 Roadway Improvement Projects

The City of Fircrest’s Draft 2026-2031 Transportation Improvement Program (TIP) identifies one improvement project near the study area. **Table 2** below provides details on the planned improvement project.

**Table 2: Six-Year Transportation Improvement Project**

Name	Location	Improvement	Cost
Alameda Avenue Active Transportation Improvement (#6)	Emerson St to Rosewood Ln	Construction of new curbs, gutters, sidewalk, and bike lane on the east side of Alameda Avenue.	\$1,900,000

The City of Fircrest plans to expand non-motorist infrastructure along Alameda Avenue from Rosewood (located just south of the school) to Emerson Street in 2029. Note there is a second project that would continue these improvements from Emerson Street to 44<sup>th</sup> Street W.

### 3.3 Transit Service

The site is not served directly by public transit. Pierce Transit operates Route 52 along Regents Boulevard in Fircrest. The nearest stop is at Regents Boulevard/Electron Way, a little over ½ mile from the school. Buses on Route 52 run from 5:45 AM to 10:03 PM on weekdays, with 30-minute headways during peak times and hourly service outside of peak times. Hourly buses operate on weekends. This route connects Tacoma Community College with the Tacoma Mall.



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Route 53 operates along Emerson Street south of the school. The nearest stop to Whittier Elementary is at Alameda Avenue/Emerson Street, about ½ mile from the school. Route 53 operates buses every 30 minutes on weekdays, from 5:30 AM to 10:29 PM. Weekend days, buses operate hourly from about 8:40 AM to 6:30 PM. This route also connects Tacoma Community College with the Tacoma Mall.

The Tacoma School District operates three to four school buses to and from Whittier Elementary School.

## 3.4 Existing Peak Hour Volumes and Travel Patterns

Traffic counts were collected covering existing school driveways and nearby intersections. Intersection turning movement data were collected in June 2025 with school in session<sup>1</sup>. The study intersections are listed below.

1. Alameda Avenue/Elm Tree Lane
2. Elm Tree Lane/North Parking Lot Access
3. Annapolis Street/Elm Tree Lane
4. Alameda Avenue/Annapolis Street
5. Alameda Avenue/West Parking Lot Access

Whittier Elementary School operates from 8:40 AM to 3:10 PM. Data were therefore collected from 7:00 to 9:00 AM and 2:00 to 6:00 PM to cover AM peak of school and street activity, the school PM peak hour, and the PM peak hour of the street. **Figure 3** illustrates volumes during the AM peak hour of the school. **Figure 4** illustrates PM peak hour of the school volumes at each existing study intersection, while **Figure 5** shows the volumes during the PM peak hour of the street. Peak times of activity were:

- AM Peak Hour: 8:00 to 9:00 AM
- PM Peak Hour of the School: 2:45 to 3:45 PM
- PM Peak Hour of the Street: 5:00 to 6:00 PM

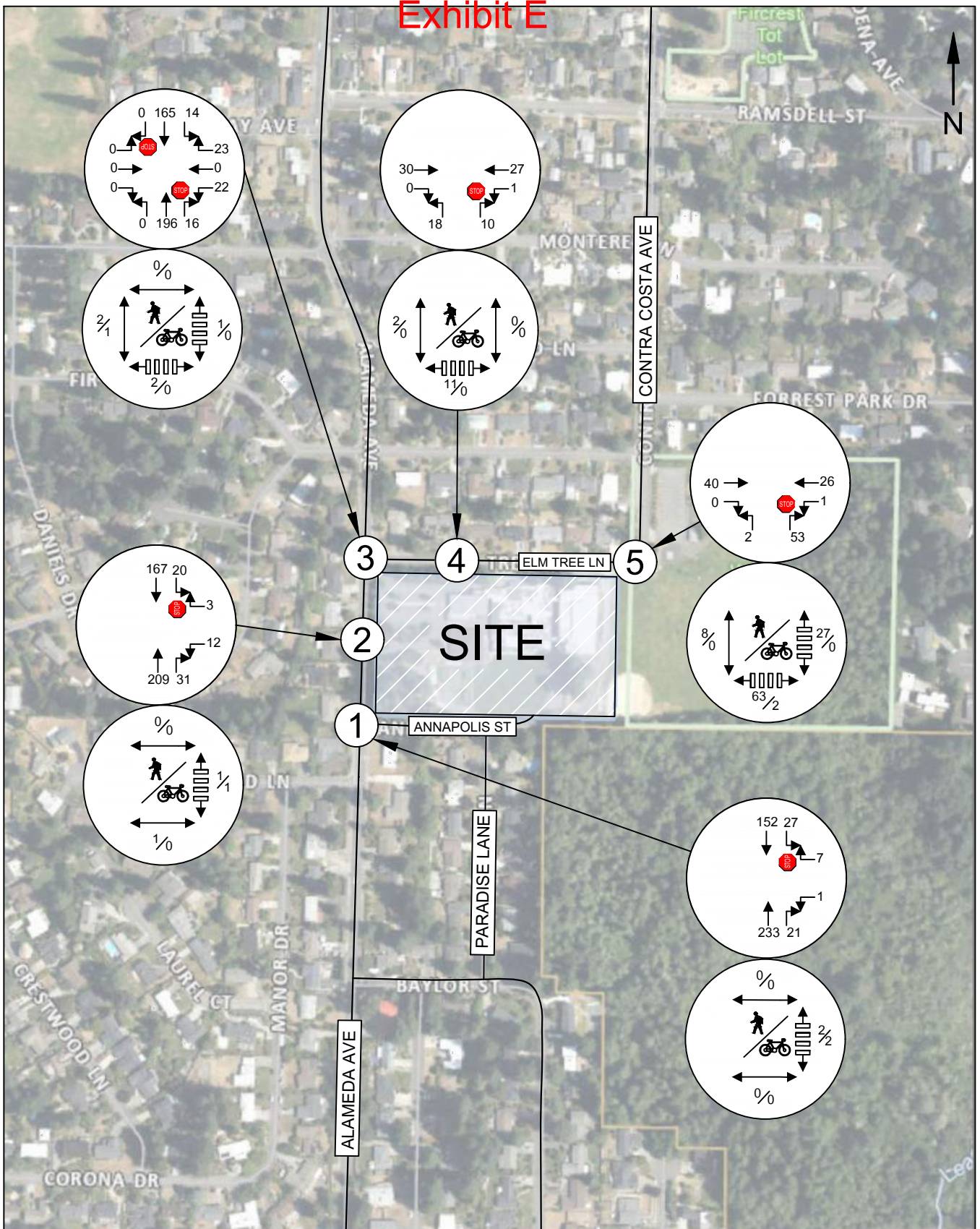
Note that a school board meeting was held at Whittier Elementary during the PM peak hour of the street. As such, the PM peak hour of the street may overstate typical traffic at the site from 5:00 to 6:00 PM. Full count sheets are attached in the appendix.

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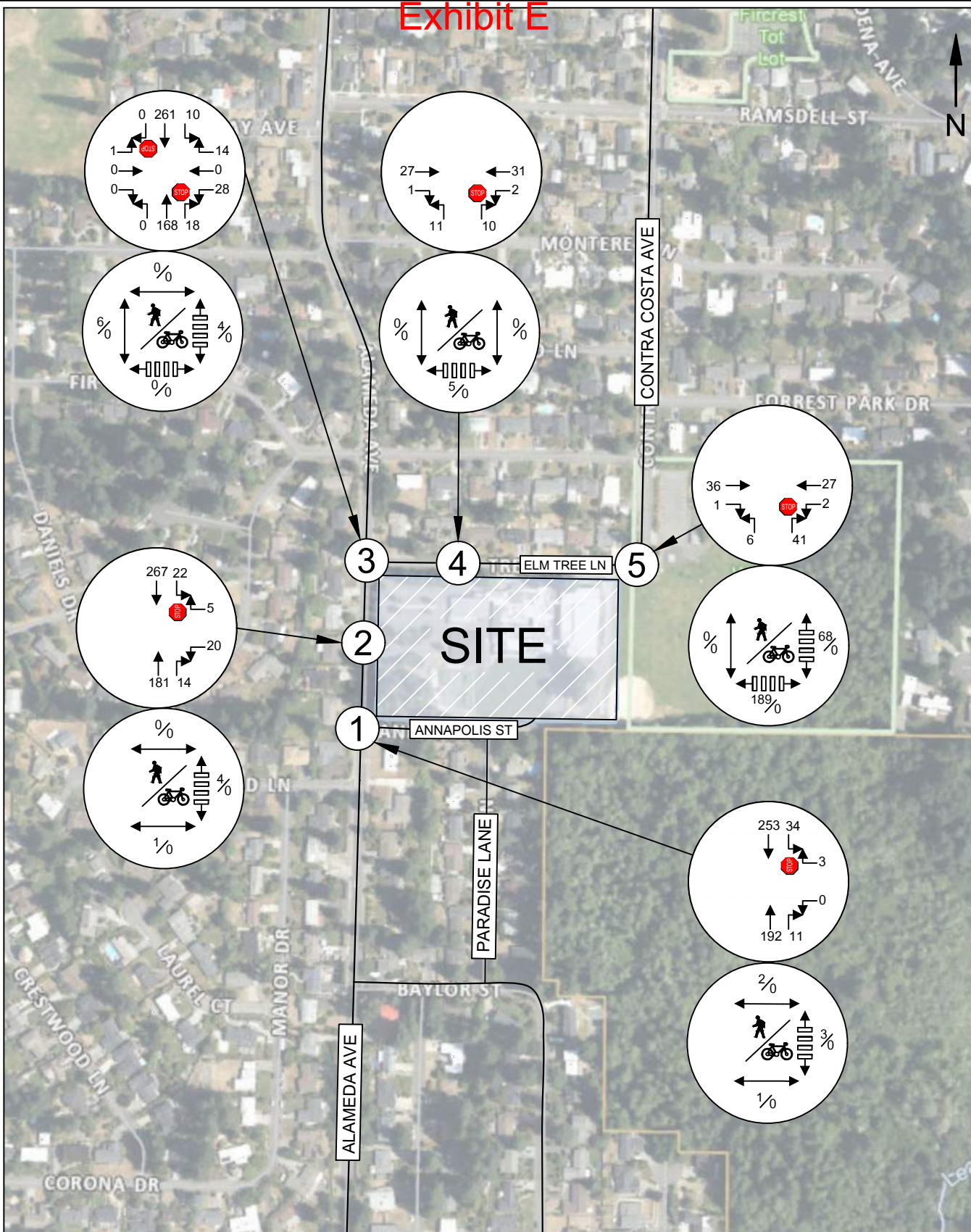
<sup>1</sup> Field data were also collected in September 2025 to capture vehicle queuing lengths along Annapolis Street. Queueing is discussed in a later section.



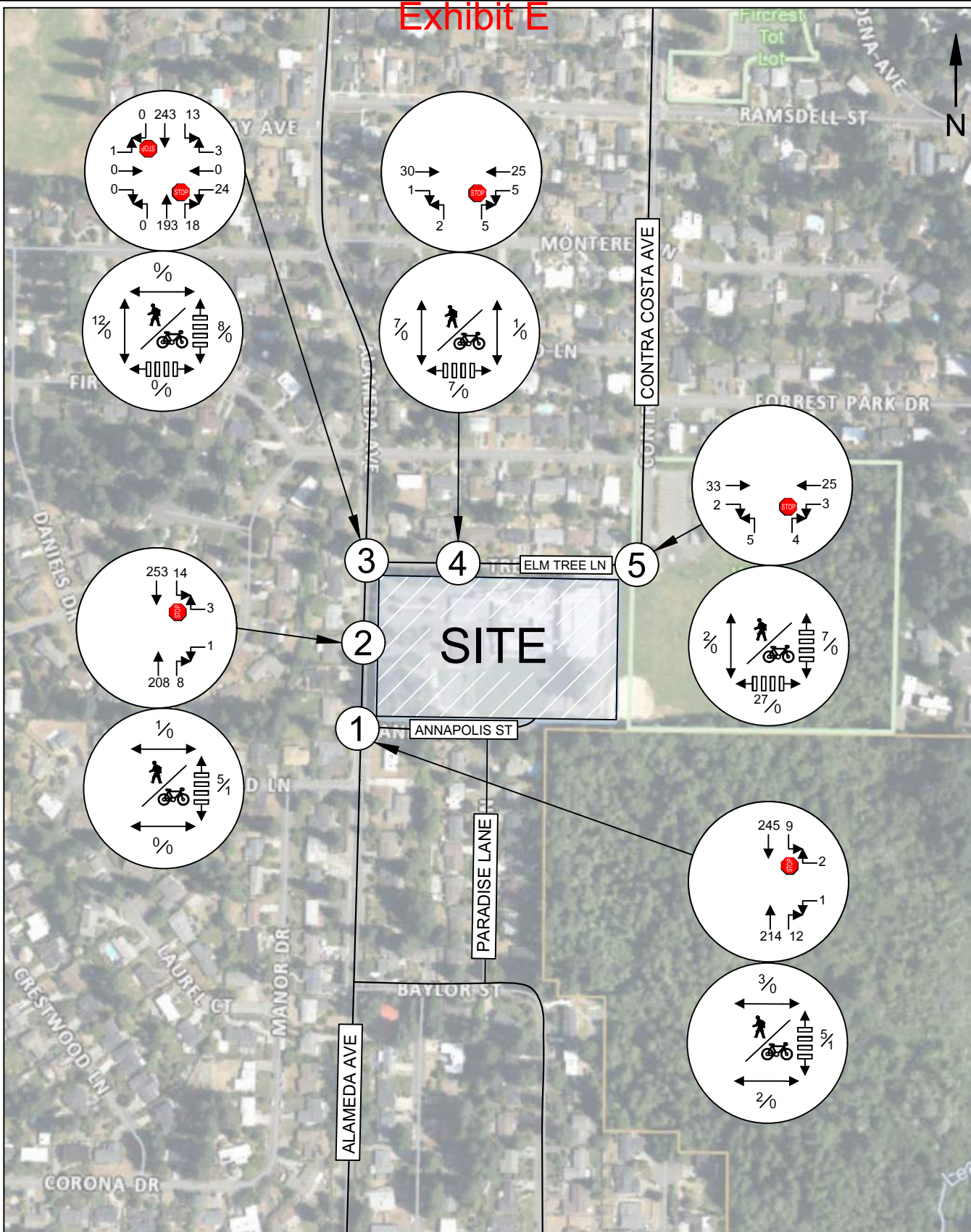
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## 3.5 Level of Service

Level of Service (LOS) rates<sup>2</sup> the quality of traffic flow and user experience, typically on a scale from A to F, where:

- **LOS A** represents free-flowing traffic with minimal delays and low congestion.
- **LOS B** indicates stable traffic flow with some minor delays.
- **LOS C** shows moderate traffic flow with noticeable delays at peak times.
- **LOS D** is high-density traffic flow with more frequent and longer delays.
- **LOS E** is near-capacity conditions with significant delays and congestion.
- **LOS F** denotes over-capacity conditions, where traffic flow breaks down, resulting in severe congestion and delays.

Level of service (LOS) calculations were performed using Synchro 12. For stop-controlled intersections, LOS is determined by the approach with the highest delay.

**Table 3** summarizes the 2025 existing LOS and delay results for the study intersections, all of which are stop-controlled on the minor approaches.

**Table 3: Existing 2025 Peak Hour Level of Service**

*Delays Given in Seconds per Vehicle*

Intersection	Approach	School AM Peak		School PM Peak		PM of Peak Street	
		LOS	Delay	LOS	Delay	LOS	Delay
1. Alameda Ave/Annapolis Street	WB	A	10.0	A	9.6	B	10.4
2. Alameda Ave/West Parking Lot	WB	B	11.5	B	12.9	B	10.6
3. Alameda Ave/Elm Tree Ln	EB/WB <sup>1</sup>	B	11.1	B	13.1	B	13.0
4. North Parking Lot/Elm Tree Ln	NB	A	8.8	A	8.8	A	8.7
5. Annapolis St/Elm Tree Ln	NB	A	9.1	B	12.7	A	9.1

<sup>1</sup> The eastbound approach experiences the greatest delays during both the AM and school PM peak hours, while in the PM peak hour of the street, the highest delays shift to the westbound approach.

The City of Fircrest has an LOS D standard for intersections. Existing 2025 peak hour levels of service are shown to operate with LOS B or better for all three study periods.

<sup>8</sup> *Signalized Intersections - Level of Service*

Level of Service	Control Delay per Vehicle (sec)
A	≤10
B	> 10 and ≤20
C	> 20 and ≤35
D	> 35 and ≤55
E	> 55 and ≤80
F	> 80

*Stop Controlled Intersections - Level of Service*

Level of Service	Control Delay per Vehicle (sec)
A	≤10
B	> 10 and ≤15
C	> 15 and ≤25
D	> 25 and ≤35
E	> 35 and ≤50
F	> 50

Highway Capacity Manual (HCM), 7th Edition



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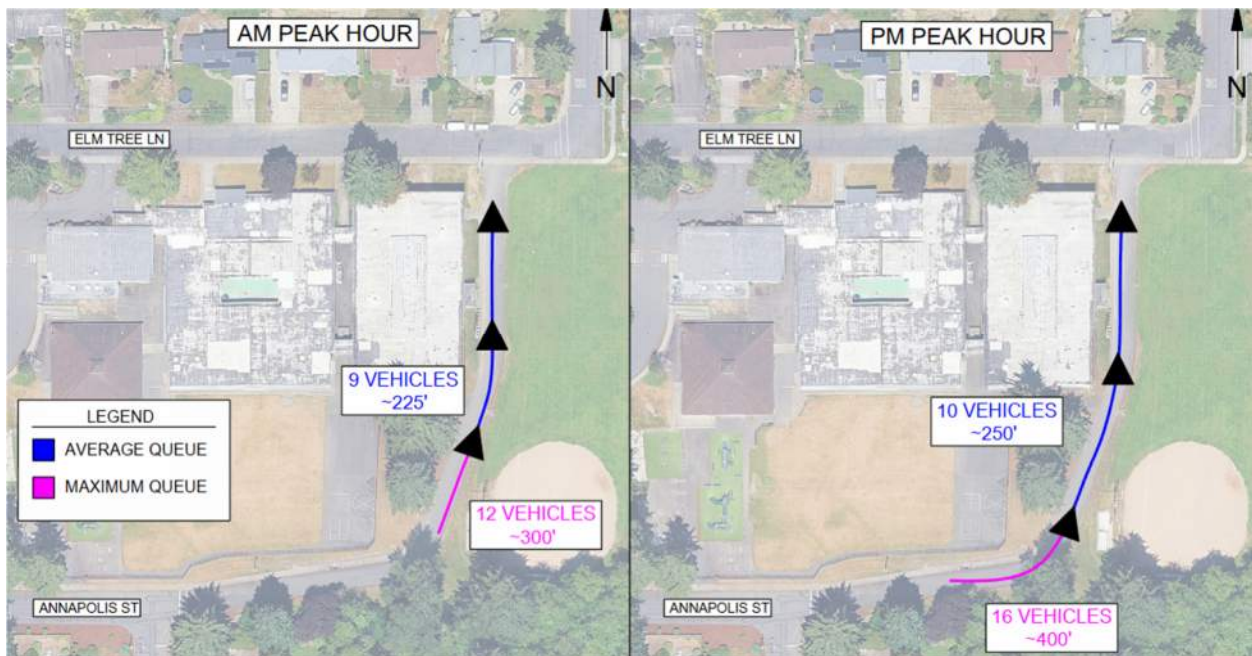
## 3.6 Queuing

While turning movement counts were collected in June 2025, additional field observations were conducted in September 2025 to review drop-off and pick-up queuing conditions. Currently, vehicles travel northbound on Annapolis Street along the east side of the school to drop off and pick up students. **Table 4** summarizes the existing average and maximum queues.

**Table 4: Existing Queuing**

Time Period	Average Queue	Max Queue
AM Peak of School	~9 vehicles	~12 vehicles
PM Peak of School	~10 vehicles	~16 vehicles

As is typical for most schools, the afternoon period exhibited higher queuing activity, with a maximum of 16 vehicles observed at 3:09 PM—just before the dismissal bell. Approximately 510 feet of queuing space is available along Annapolis Street, extending from south of Elm Tree Lane to the existing crosswalk on the east side of Paradise Lane. The 16 queued vehicles occupied roughly 400 feet, and no spillover beyond Paradise Lane was observed. However, some parents and guardians were also observed using on-street parking along Paradise Lane and Contra Costa Avenue, as well as the Whittier Park parking lot.



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## 3.7 Parking

Whittier Elementary provides 53 surface parking stalls within the on-site lot. Parking utilization was evaluated by recording vehicle entries and exits in 15-minute intervals between 7:00 AM and 6:00 PM. The analysis found a peak utilization rate of 89% (47 vehicles) between 3:00 and 3:15 PM. On average, the lot was approximately half occupied, with demand increasing during the morning and afternoon drop-off and pick-up periods. **Figure 6** below illustrates parking utilization trends throughout the study day in September.

**Figure 6: On-Site Parking Utilization**



Parking utilization was also observed at the nearby Fircrest Park lot, located just northeast of the school. Parents occasionally used this lot for short-term drop-off and pick-up, though it primarily served park visitors. The 32-stall lot reached a maximum utilization of 69% (22 vehicles) during the same period as the on-site peak, with approximately 13 vehicles likely associated with school activity. For most of the day, utilization remained below 50%.



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## 3.8 Non-Motorist Activity and Infrastructure

Non-motorized traffic was recorded during the three study-hour count periods. At the Annapolis Street/Elm Tree Lane intersection, approximately 98 pedestrians were observed during the AM peak hour and 257 pedestrians during the PM school peak hour. Note that some of these observations represent the same individuals crossing in multiple directions (east-west and north-south).

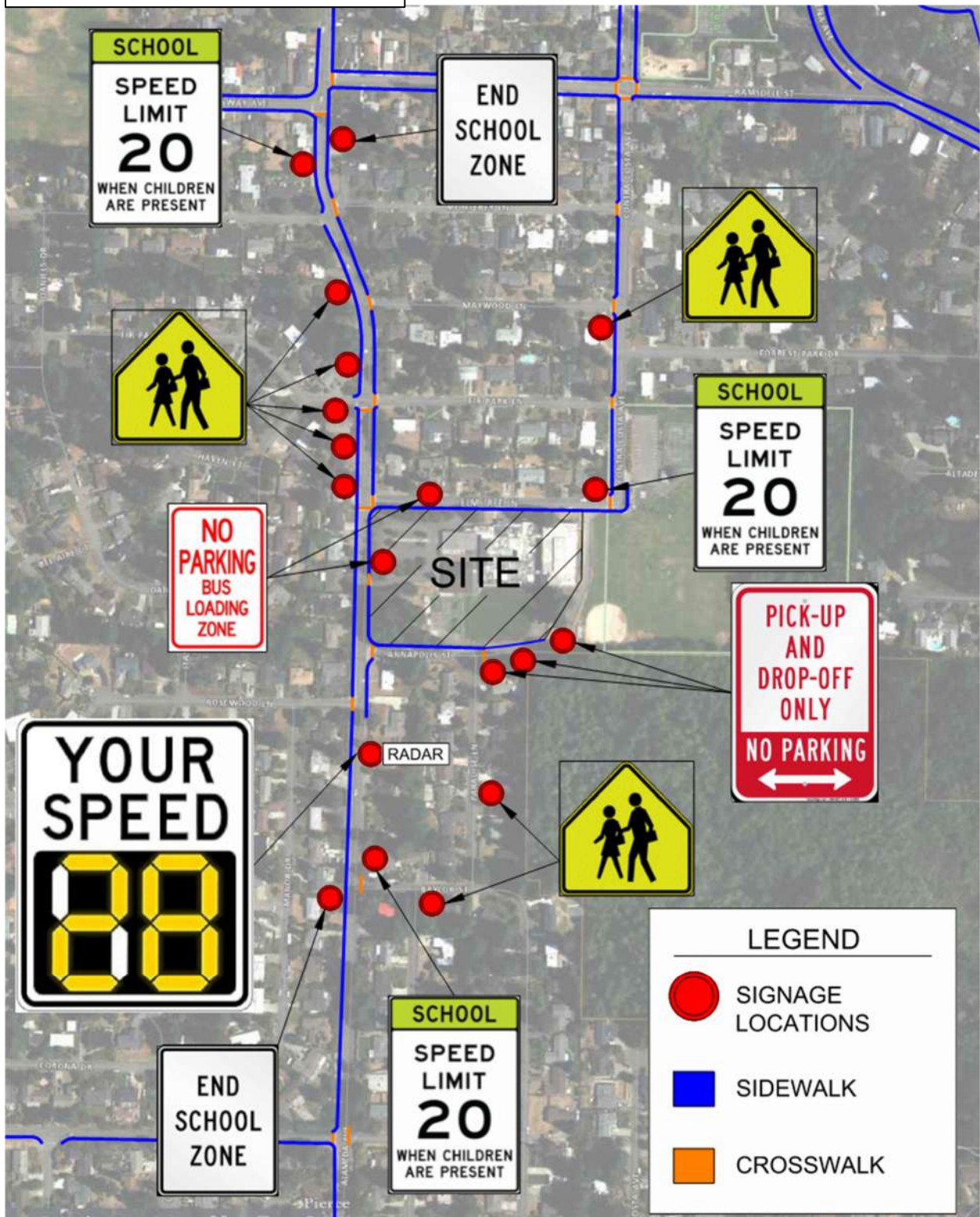
This is the busiest intersection for pedestrians; the other study intersections have fewer, ranging from 2 to 13 per study hour. Figures 3 through 5 show the numbers of pedestrians and bicyclists observed around the school during the AM and PM peak hours of study. Observations suggest that some parents and guardians currently park along Contra Costa Avenue or at the park parking lot and walk students to and from the school, while others access the campus on foot from nearby residences. Crossing guards stop vehicular traffic at the Elm Tree Lane/Annapolis Street intersection to facilitate pedestrian crossings.

**Figure 7** on the following page illustrates the nearby school signs and crossing signs along with sidewalk and crosswalk locations.



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Figure 7: Existing School Signs



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## 3.9 Collision History

Crash records for the five most recent years (2020 through 2024) were obtained from Washington State Department of Transportation (WSDOT). There were two incidents reported in the study area in that time. These are summarized below.

**Collision #1: Alameda Avenue south of Rosewood Lane:** This collision occurred over the summer (7/24/23) at 2:09 AM. A driver traveling northbound on Alameda Avenue struck a parked, unoccupied vehicle about 50 feet south of Rosewood Lane. The driver was noted as lost in thought or daydreaming. There were no apparent injuries related to this incident.

**Collision #2: Elm Tree Lane east of Alameda Avenue:** On February 29, 2024, at 5:38 PM, a driver traveling on Elm Tree Lane struck a parked vehicle about 200 feet east of Alameda Avenue. The report does not state which direction the driver was headed. The vehicle struck was unoccupied. The driver behavior was listed as "other contributing circumstance." The weather condition was raining, and the pavement was wet. No injuries were reported.



## 4. FORECAST TRAFFIC DEMAND AND ANALYSIS

### 4.1 Trip Generation

Trip generation for the school was estimated using the *ITE Trip Generation Manual, 12<sup>th</sup> Edition*. ITE average rates applied, using data for Land Use Code (LUC) 520 - Elementary School. The updated school will have a maximum enrollment capacity of 380 students. The school currently serves approximately 320 students. Since this traffic is already accounted for in the existing conditions, the trip generation assessment was based on the expanded capacity of 60 students. **Table 5** summarizes the average weekday daily trips (AWDT) as well as the school's AM and PM peak hours, and the PM peak hour of the adjacent street network, defined as the busiest hour between 4:00 and 6:00 PM.

**Table 5: Project Trip Generation**

Land Use	Additnl Students	AWDT	School AM Peak			School PM Peak			4-6 PM Peak Street		
			In	Out	Total	In	Out	Total	In	Out	Total
Elementary School (LUC 520)	60	136	24	20	<b>44</b>	12	14	<b>26</b>	5	5	<b>10</b>

With the addition of 60 potential students in the future school, a total of 136 new daily vehicle trips are expected, with 44 during the AM peak hour, 26 during the school PM peak hour, and 10 during the PM peak hour of the street.

### 4.2 Trip Redistribution and Assignment

**Vehicle Routing:** Student drop-off and pick-up activity will continue along Annapolis Street on the east side of the school; however, the traffic flow will be reversed under the updated site plan, with vehicles queuing southbound. The revised layout will provide approximately 565 feet of queuing capacity, a slight increase from the existing 510 feet. **Figure 8** illustrates the new drop-off and pick-up route.

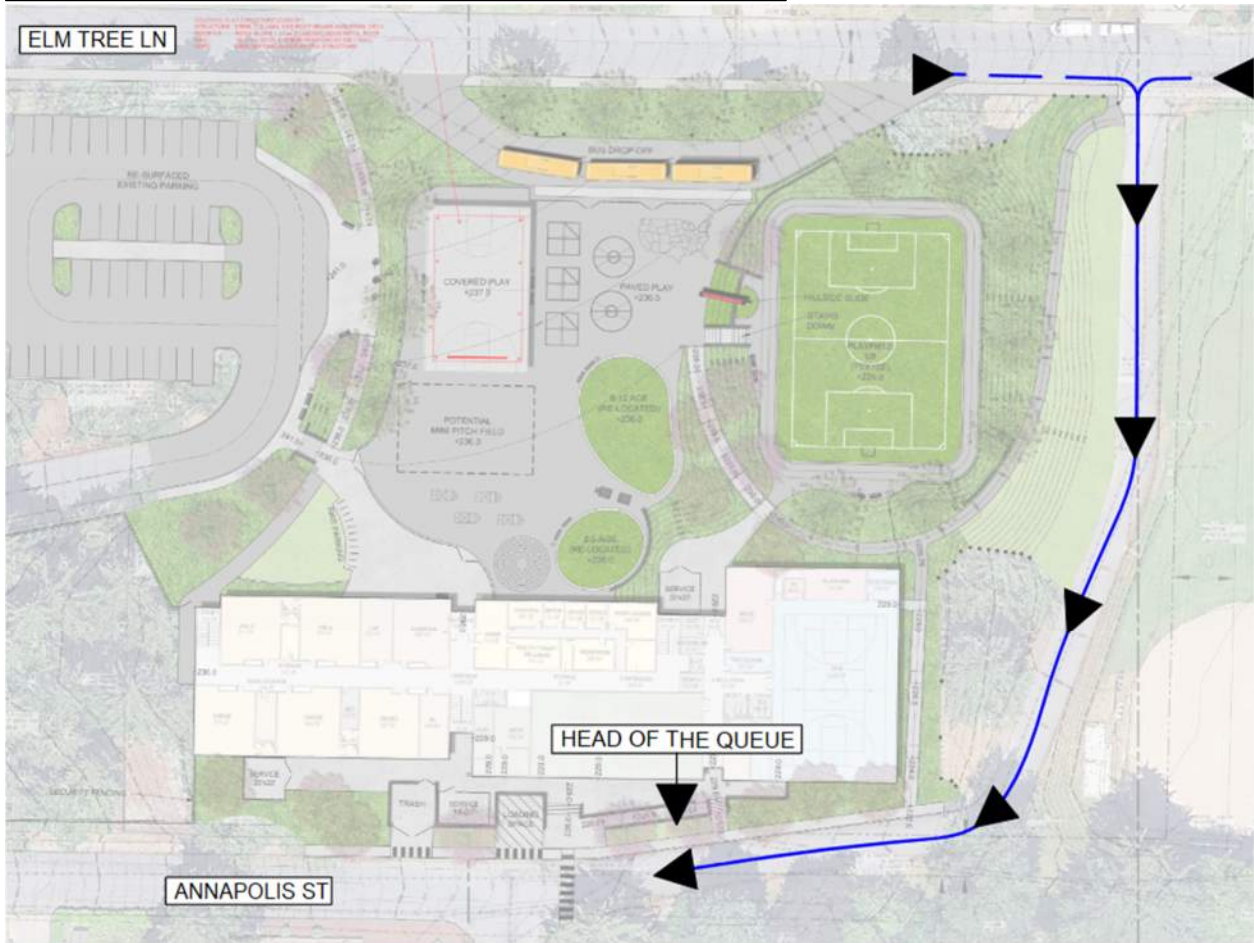
**Bus Routing:** Currently, buses queue eastbound along Elm Tree Lane. Under the proposed site plan, a new bus loop accessed from Elm Tree Lane would eliminate the need for buses to park within the public right-of-way. The school typically operates three to four buses, which is expected to remain consistent under future conditions.

**Existing Traffic:** Because of the changes to the site access and circulation, existing traffic was redistributed to the access and egress patterns.



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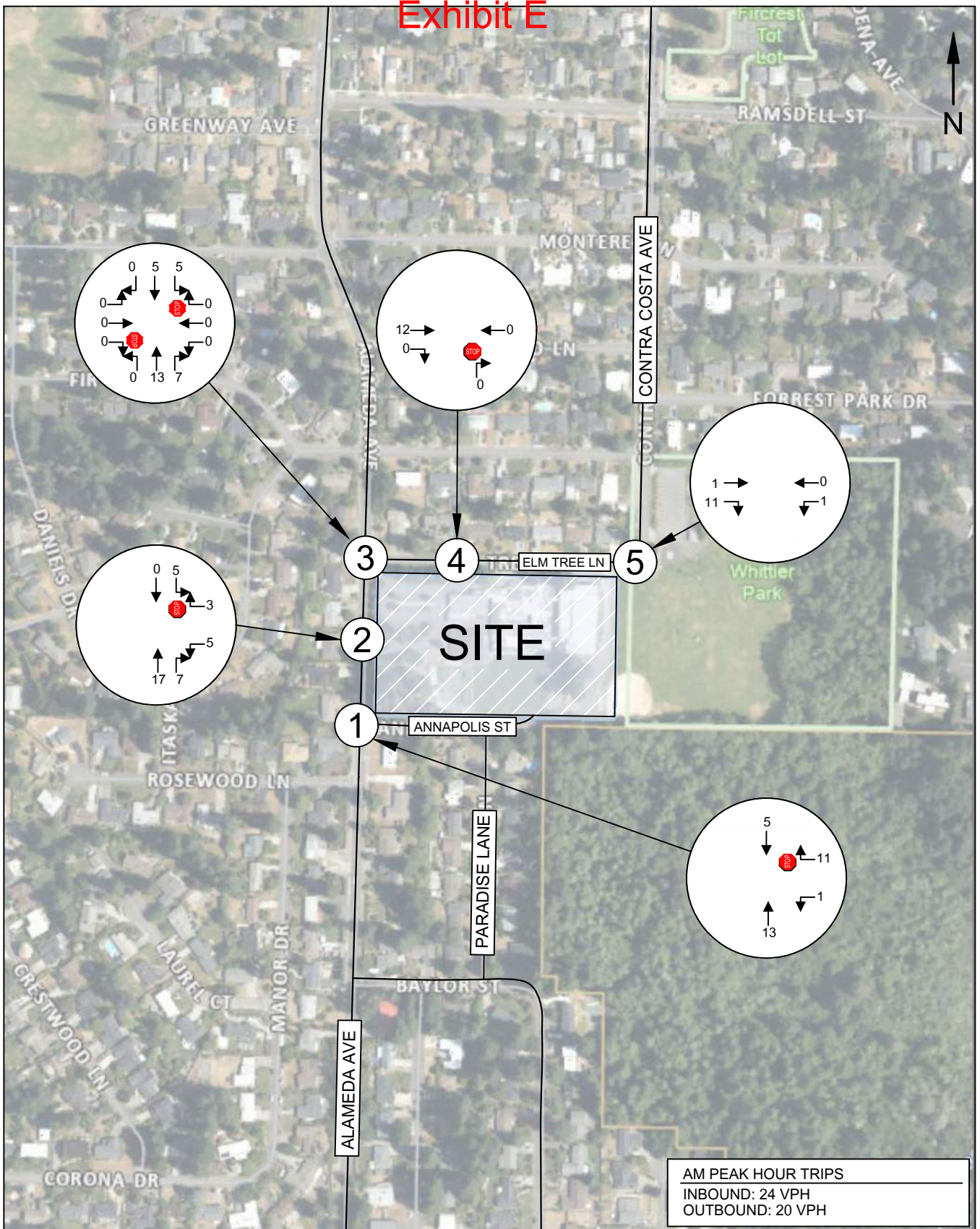
**Figure 8: Proposed Drop-Off and Pick-Up Route**



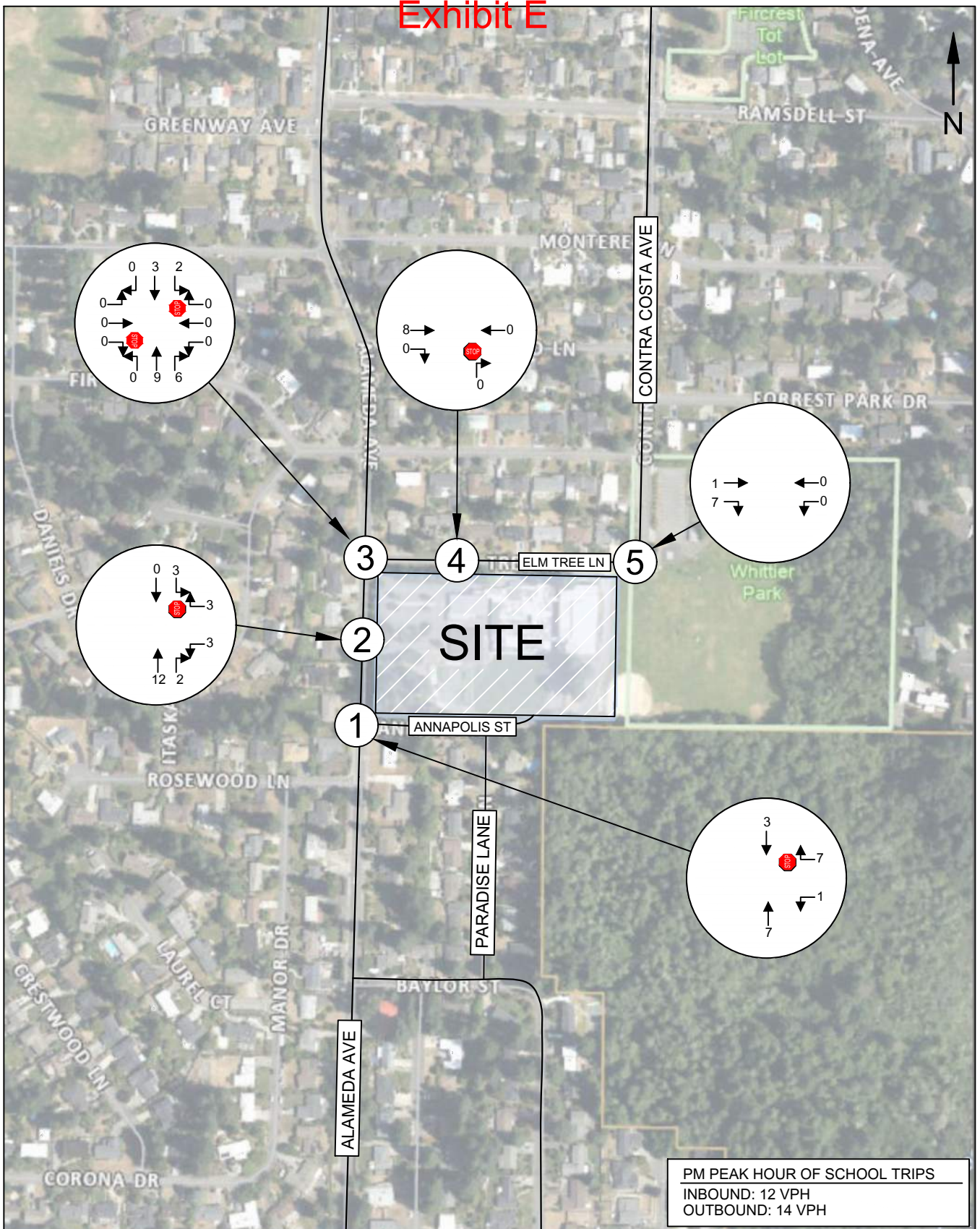
Trip assignments for the new project trips are illustrated in **Figures 9, 10, and 11**.



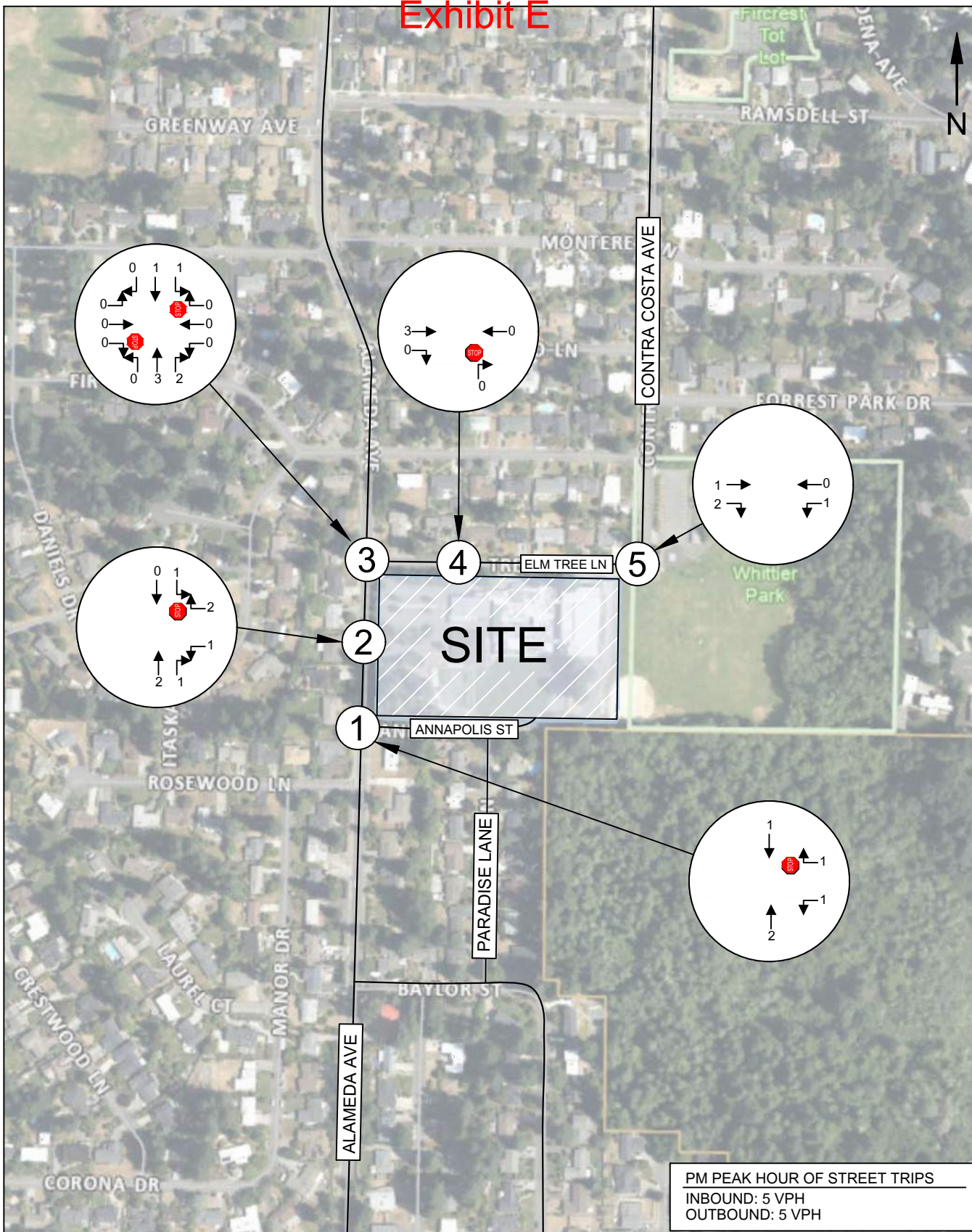
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## 4.3 Future Peak Hour Volumes

A three-year horizon of 2028 was used to assess future conditions with project buildout. This assumes Whittier Elementary would be operating at a full capacity of 380 students. Forecast 2028 peak hour background volumes were derived by applying a one percent compound annual growth rate per year to the existing volumes.<sup>3</sup>

Forecast 2028 volumes without project for AM and PM peak hours are illustrated in **Figures 12, 13, and 14**<sup>4</sup>. No pipeline projects are expected in the study area. Forecast 2028 volumes with project for the AM and PM peak hours are illustrated in **Figures 15, 16, 17**. This includes redistribution of the existing school traffic due to the changes in access with the project in place.

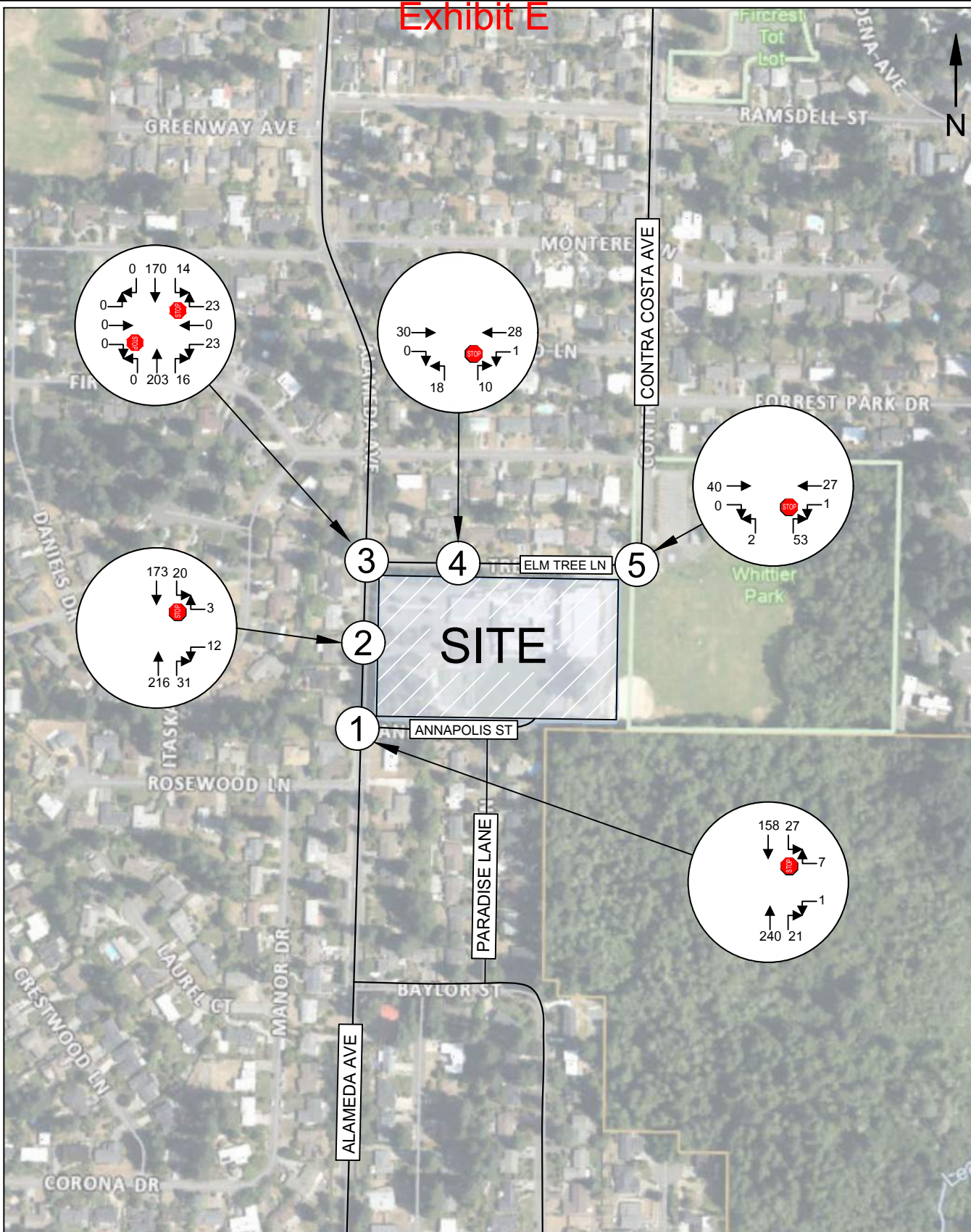
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<sup>3</sup> Confirmed with City of Fircrest.

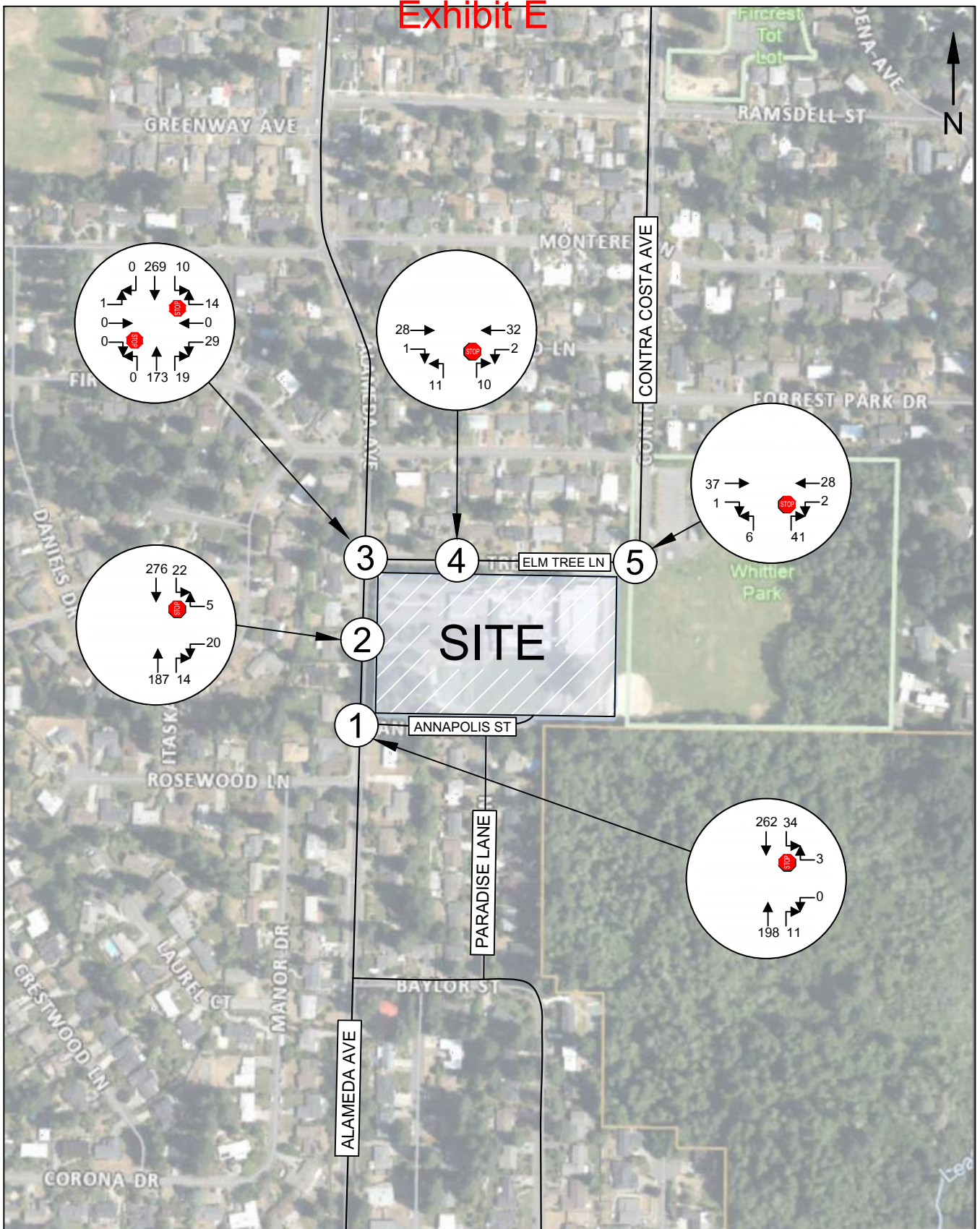
<sup>4</sup> The future without project scenario analyses assume the same traffic patterns as today.



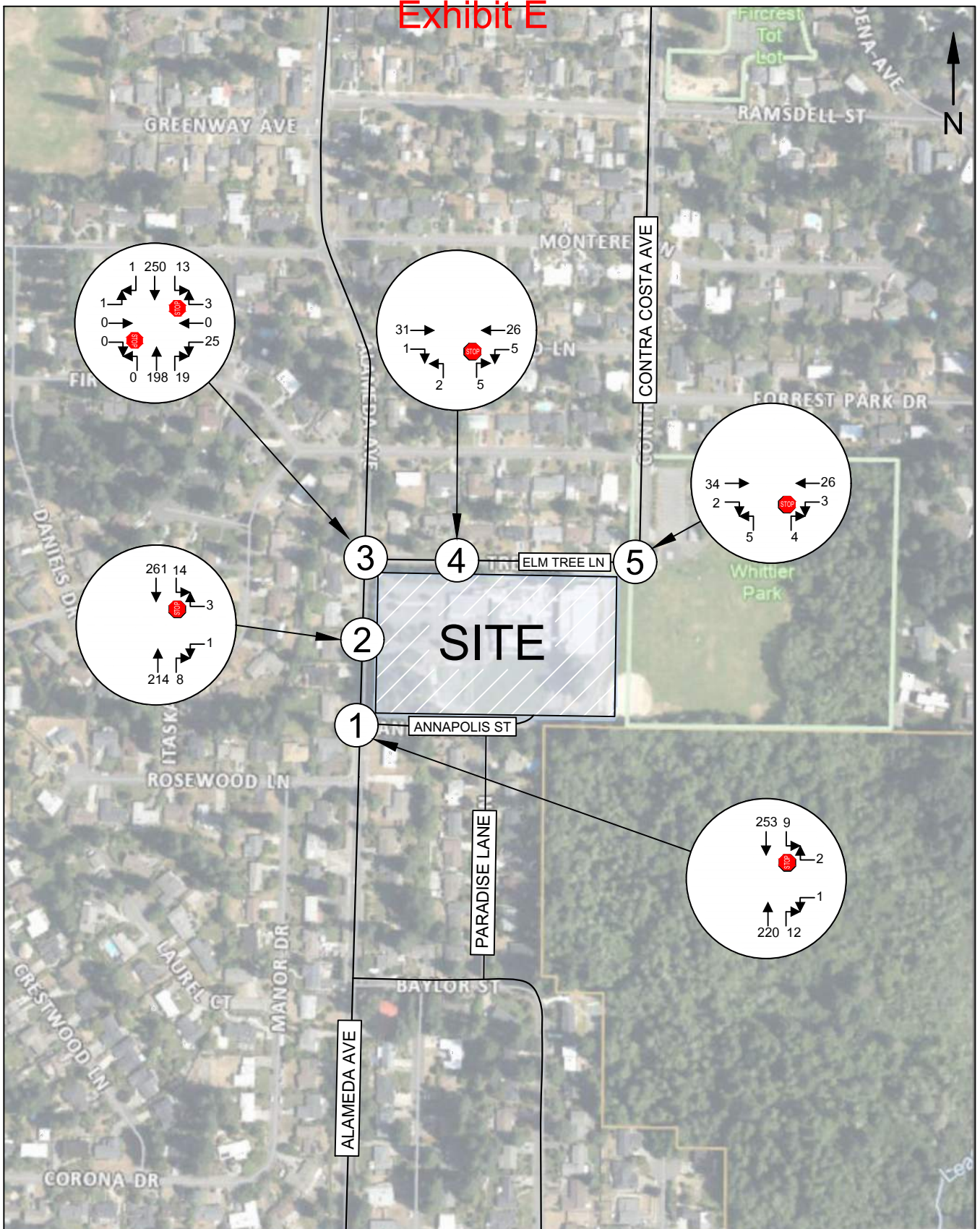
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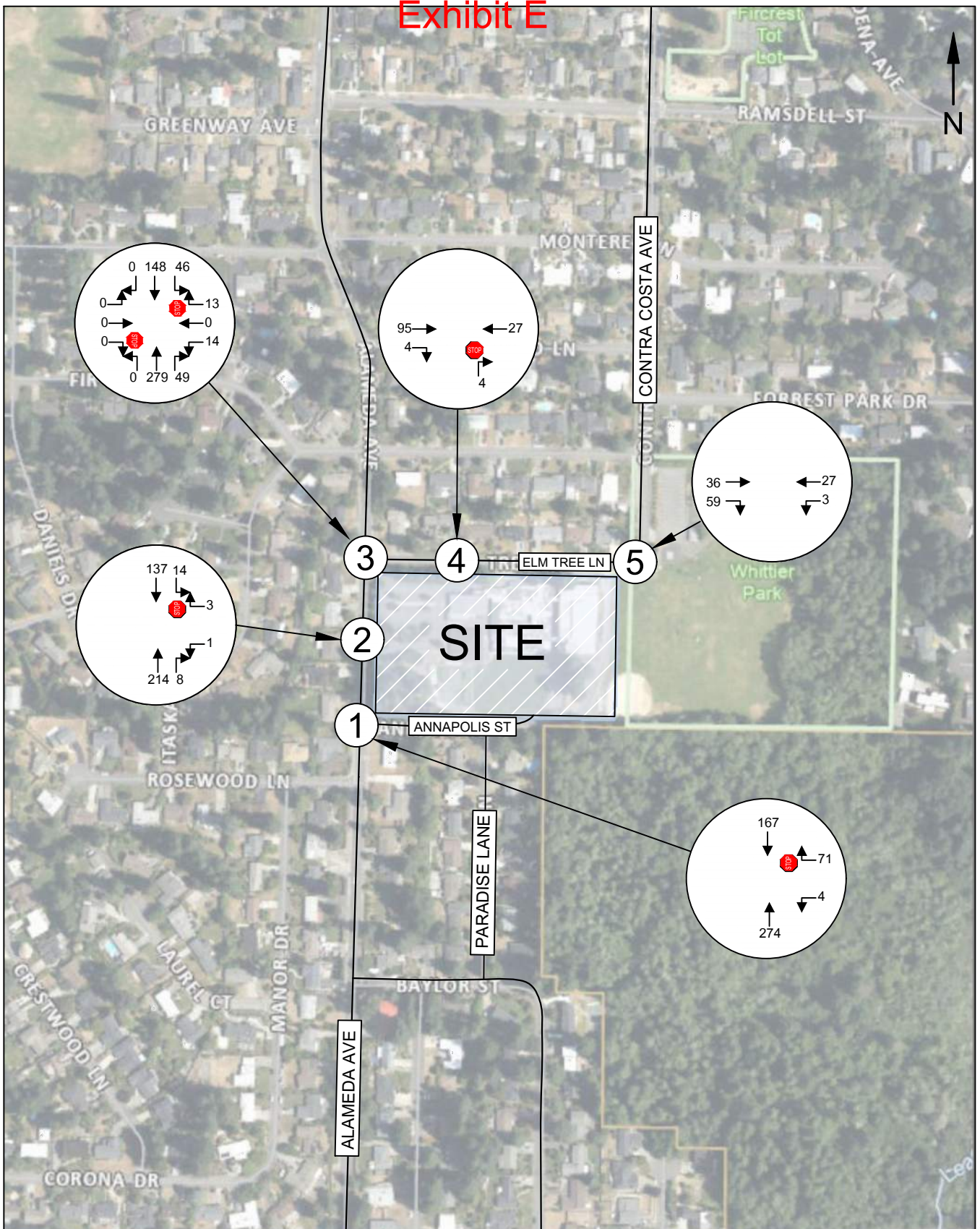
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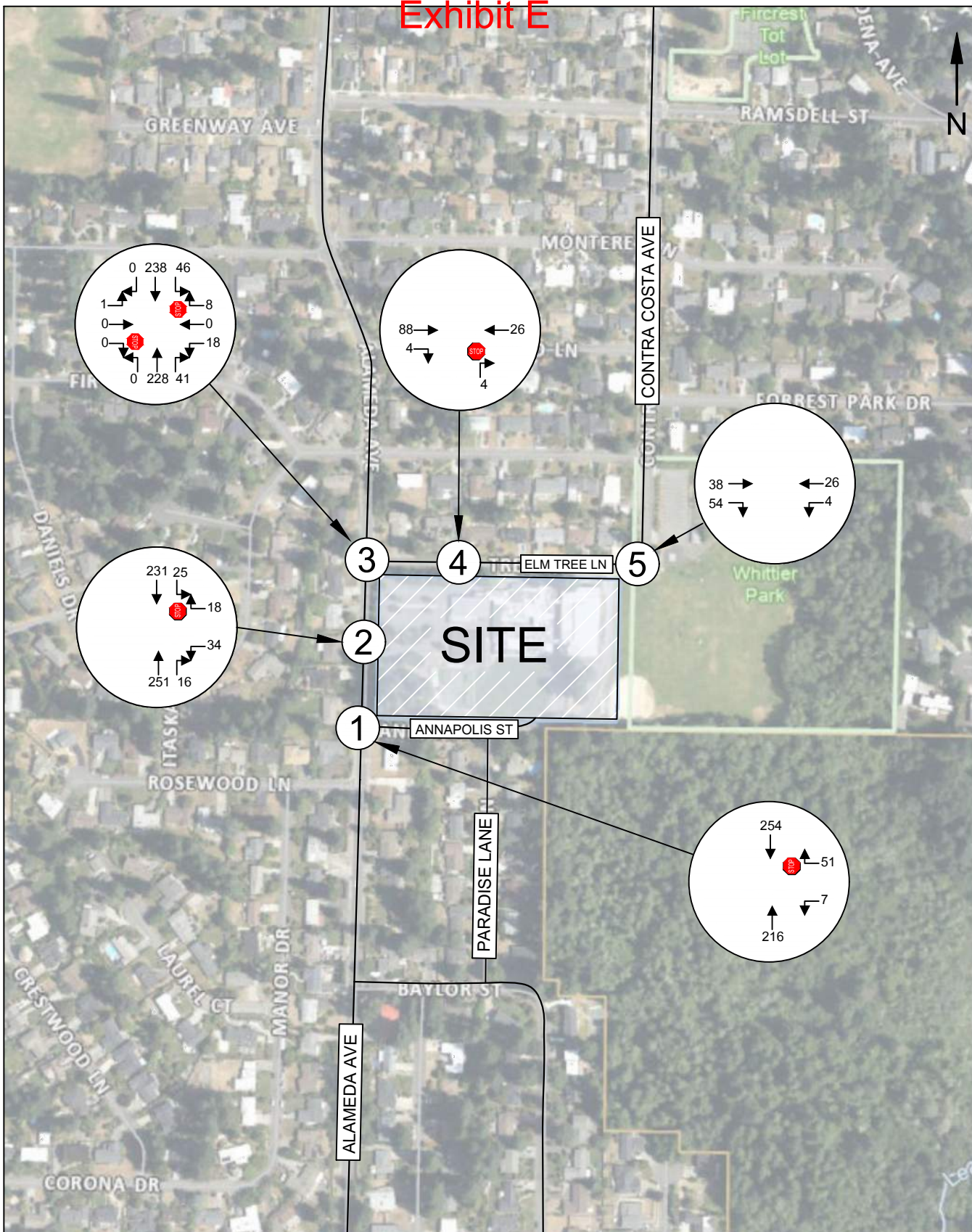
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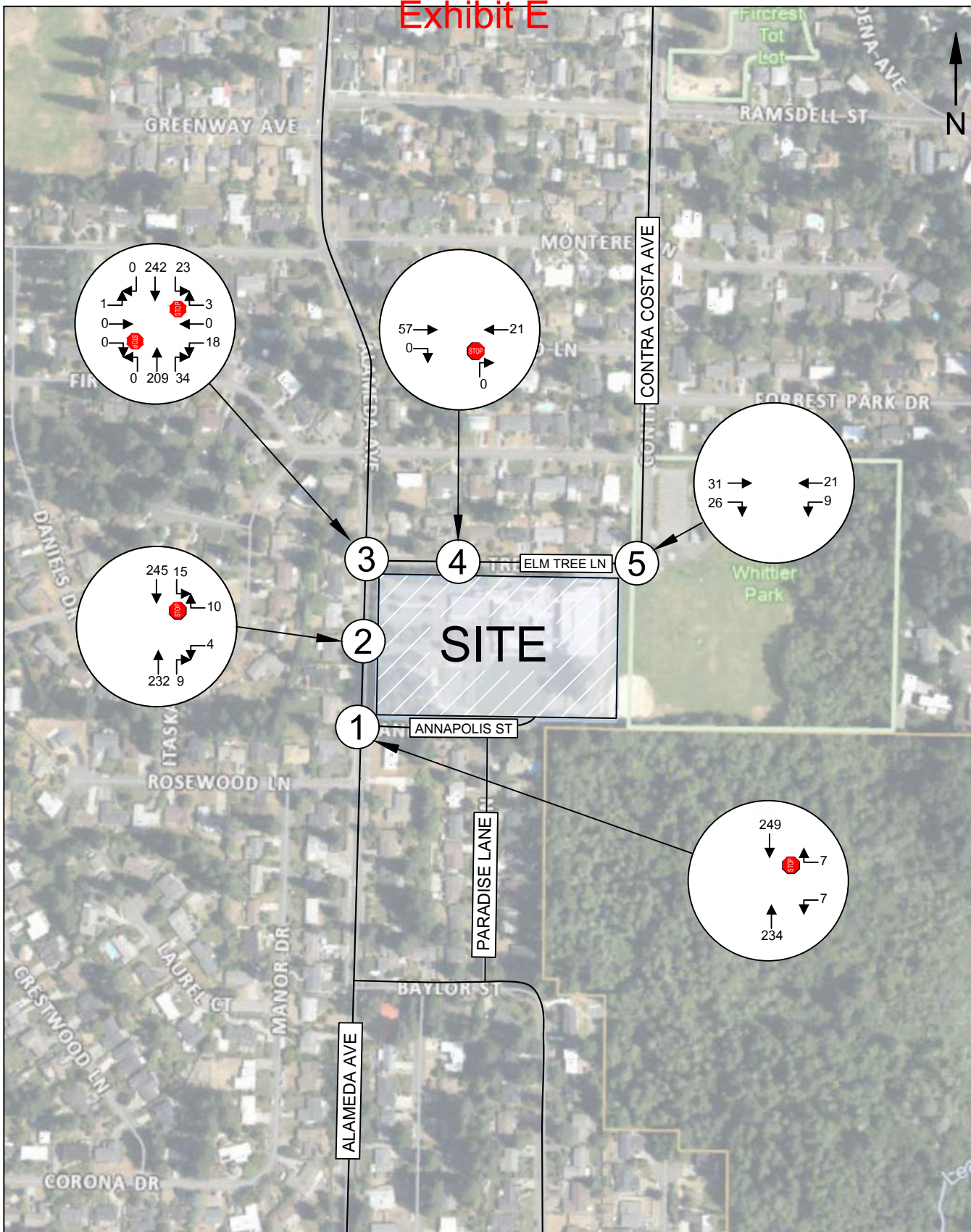
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## 4.4 Future Level of Service

Level of service analyses were conducted for future peak hour conditions both without and with the additional school-related trips added to the study intersections. The resulting delays and levels of service under are summarized below in **Table 6**. It should be noted that the Elm Tree Lane/North Parking Lot driveway will serve only buses with the project in place. Since no bus activity is expected during the PM peak hour of the adjacent street network, no delay is reported for that period.

**Table 6: Forecast 2028 Weekday Peak Hour Level of Service**

*Delays Given in Seconds per Vehicle*

Intersection	Approach	Peak Hour	Without Project		With Project	
			LOS	Delay	LOS	Delay
1. Alameda Ave/ Annapolis Street	WB	AM Peak	B	10.0	B	12.0
		PM School	A	9.7	B	10.6
		PM Street	B	10.5	B	10.9
2. Alameda Ave/ West Parking Lot	WB	AM Peak	B	11.6	B	14.3
		PM School	B	13.1	B	13.6
		PM Street	B	10.6	B	11.0
3. Alameda Ave/ Elm Tree Ln	WB/EB	AM Peak	B	11.3	B	14.2
		PM School	B	13.3	C	15.2
		PM Street	B	13.2	B	13.5
4. Elm Tree Lane/ North Parking Lot	NB	AM Peak Hour	A	8.8	B	10.2
		PM School	A	8.8	B	10.2
		PM Street	A	8.7	A	0.0
5. Annapolis St/ Elm Tree Ln	NB/WB	AM Peak	A	9.1	A	0.9
		PM School	A	12.8	A	1.1
		PM Street	A	9.1	A	2.3

Operations improve at the Elm Tree Lane/Annapolis Street intersection in with project conditions because Annapolis Street becomes one way southbound in the future along the east side of the school. With no northbound exiting trips, delays decrease due to fewer conflicts. The approach with the highest delay also changes, from northbound to westbound.

Forecast 2028 AM and PM peak hour operations are projected to be LOS C or better. The study intersections continue to meet City of Fircrest standards in future conditions. Overall, the project is not expected to create a significant impact.



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## 4.5 Queuing

As discussed in Section 3.6, the current maximum queue of 16 vehicles with 320 students represents a vehicle-to-student ratio of approximately 5%. With the addition of 60 students, queuing demand may increase by about three vehicles, resulting in a projected maximum of 19 vehicles (approximately 475 feet). The proposed configuration provides roughly 565 feet of available queuing space, indicating that the future maximum queue should remain within the available length. Vehicles are not expected to spill back onto Elm Tree Lane. This assumes that some parents and guardians would continue to utilize street parking and on-site parking stalls for pick-up and drop-off activity.

## 4.6 Parking

The existing school campus provides 53 parking stalls, and the proposed redevelopment plans will revise this to 50 stalls. Parking requirements for schools are established in the Fircrest Municipal Code, Chapter 22 (Section 22.60.003). A summary of the code-required parking for the proposed school is shown in **Table 7**.

**Table 7: City of Fircrest Parking Requirements - Future School**

Land Use	Per Preschool Student Standard	Per Elementary Student Standard	Per Classroom Standard
Elementary	1 stall/6 students	1 stall/50 students	1 stall/classroom

Applying the parking requirements results in the following minimum required parking supply:

- Students - Preschool (36) = 6 stalls
- Students - Elementary (344) = 7 stalls
- Classrooms (21) = 21 stalls
- Total = 34 parking spaces

Note that, per City of Fircrest standards, the maximum allowable parking stall supply is 51 stalls. The project is proposing 50 parking stalls.

Parking utilization counts showed an average demand of about 28 vehicles, with a peak of 47 vehicles—exceeding the minimum code requirement. Parking demand may continue to rise as student enrollment grows. To evaluate potential future parking needs, the *Institute of Transportation Engineers (ITE) Parking Generation Manual, 6th Edition* was reviewed. Using Land Use Code 520 - Elementary School, estimated parking demands were developed and are summarized in **Table 8**.



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**Table 8: ITE Parking Demand**

*Rates Expressed in Parking Stalls per Students*

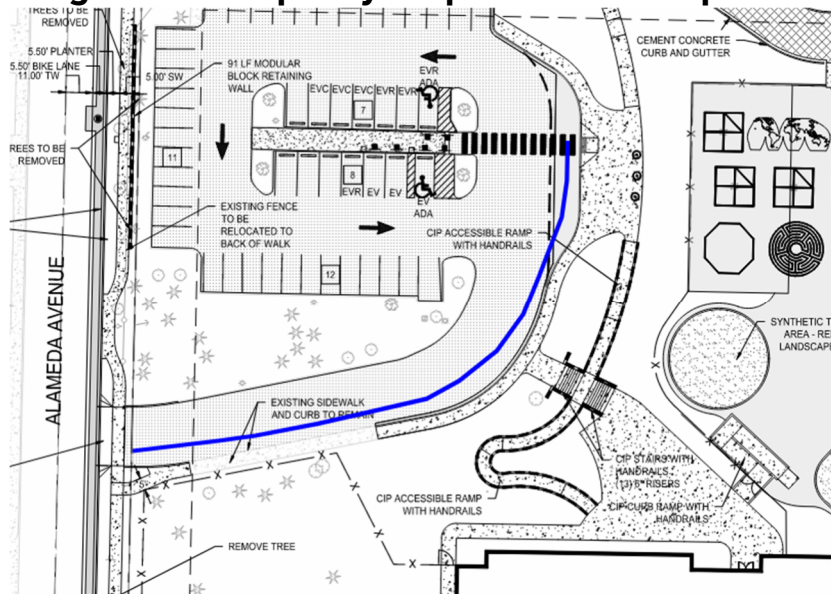
Land Use	Students	Average Rate	Parking Demand	Proposed Supply
Elementary School	380	0.14	53 veh	50 spaces

For an elementary school with 380 students, ITE data indicates a parking demand of approximately 53 stalls. The updated site configuration will provide 50 stalls, which is expected to accommodate typical daily demand; however, parking demand may temporarily exceed supply during peak drop-off and pick-up periods.

## 4.7 Queuing and Parking - Construction Final Months

The new school building is anticipated to open in Fall 2027, with construction of the fields and play areas continuing until January 2028. During this final construction phase, Annapolis Street will be unavailable for parent and guardian drop-off and pick-up. These activities will temporarily take place within the on-site parking lot shown below in **Figure 18**, which provides approximately 230 feet of queuing capacity—less than the existing demand of about 400 feet. As such, the school should proactively inform parents of these temporary conditions and encourage alternative transportation options such as walking, biking, carpooling, or parking on nearby streets and walking to school to minimize impacts on Alameda Avenue.

**Figure 18: Temporary Drop-Off and Pick-Up Area**



## 5. CONCLUSIONS

The Whittier Elementary School project is intended to rebuild the existing school and create capacity for up to 60 additional students. The school is located in and serves Fircrest and is part of the Tacoma School District. The future site will move the building to the south and create new fields and play areas. On-site drop off and pick up activity will remain along Annapolis Street, but queueing direction will change from northbound to southbound.

Data were collected and operations evaluated for the AM peak hour, the school's PM peak hour, and the street's PM peak hour. All intersections operated at LOS B or better under existing conditions. A review of the past five years of collision data identified two reported incidents, neither involving injuries. During afternoon pick-up, a maximum queue of 16 vehicles was observed along Annapolis Street, with additional activity on adjacent streets and within the on-site parking lot. The lot, which provides 53 stalls, reached a peak utilization of 89% between 3:00 and 3:15 PM, averaging around 50% occupancy throughout the day.

With an additional 60 students, the project is estimated to generate 136 average weekday daily trips with 44 AM peak hour trips, 26 PM peak hour of school trips, and 10 PM peak hour of street trips. A three-year horizon (2028) was used to assess full buildout conditions. Because of the change in drop-off and pick-up activity, all existing project trips were redistributed for the forecast with project scenario. Forecast 2028 LOS are projected at LOS C or better for all three scenarios, without or with the project. The City of Fircrest level of service standards will be met with the project in place.

Assuming the new school at full-capacity, vehicle queuing on Annapolis Street is estimated at 19 vehicles (approximately 475 feet) in future conditions. The proposed configuration provides roughly 565 feet of available queuing space, so the future maximum queue should be accommodated within the available length. The updated site configuration will provide 50 stalls, accommodating typical daily demand.

Site redevelopment will include frontage improvements such as a 5.5-foot bike lane on the east side of Alameda Avenue, new curb, gutter, and sidewalk along the school frontages on Alameda Avenue and Elm Tree Lane, and pedestrian curb bulbs at the intersections of Alameda Avenue with Elm Tree Lane and Annapolis Street. During the final three months of construction, the Annapolis Street drop-off and pick-up lane will not be available. These activities will temporarily take place within the on-site parking lot. That lot provides approximately 230 feet of queuing capacity—less than the existing demand of about 400 feet. As such, the school should proactively



## Exhibit E

During the final three months of construction, the Annapolis Street drop-off and pick-up lane will not be available. These activities will temporarily take place within the on-site parking lot. That lot provides approximately 230 feet of queuing capacity—less than the existing demand of about 400 feet. As such, the school should proactively inform parents of these temporary conditions and encourage alternative transportation options such as walking, biking, carpooling, or parking on nearby streets and walking to school to minimize impacts on Alameda Avenue.

Please feel free to contact me should you have any questions.

Aaron Van Aken, P.E., PTOE



# Exhibit E

## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

### *APPENDIX* Count Sheets



# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623c  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Driveway Southbound				Elm Tree Ln Westbound				School Access Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00 AM	0	0	0	0	0	1	1	2	0	0	0	0	0	2	0	2	4
07:15 AM	0	0	0	0	0	1	1	2	0	0	0	0	0	1	0	1	3
07:30 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	2	0	2	4
07:45 AM	0	0	1	1	0	0	0	0	0	0	0	0	0	2	0	2	3
<b>Total</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>4</b>	<b>2</b>	<b>6</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>7</b>	<b>0</b>	<b>7</b>	<b>14</b>
08:00 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	3	0	3	5
08:15 AM	0	0	0	0	0	4	1	5	1	0	2	3	0	7	0	7	15
08:30 AM	0	0	0	0	0	12	0	12	8	0	13	21	0	11	0	11	44
08:45 AM	0	0	0	0	0	10	0	10	1	0	3	4	0	7	0	7	21
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>28</b>	<b>1</b>	<b>29</b>	<b>10</b>	<b>0</b>	<b>18</b>	<b>28</b>	<b>0</b>	<b>28</b>	<b>0</b>	<b>28</b>	<b>85</b>
<b>Grand Total</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>32</b>	<b>3</b>	<b>35</b>	<b>10</b>	<b>0</b>	<b>18</b>	<b>28</b>	<b>0</b>	<b>35</b>	<b>0</b>	<b>35</b>	<b>99</b>
Apprch %	0	0	100		0	91.4	8.6		35.7	0	64.3		0	100	0		
Total %	0	0	1	1	0	32.3	3	35.4	10.1	0	18.2	28.3	0	35.4	0	35.4	
Passenger +	0	0	1	1	0	31	3	34	9	0	18	27	0	29	0	29	91
% Passenger +	0	0	100	100	0	96.9	100	97.1	90	0	100	96.4	0	82.9	0	82.9	91.9
Heavy	0	0	0	0	0	1	0	1	1	0	0	1	0	6	0	6	8
% Heavy	0	0	0	0	0	3.1	0	2.9	10	0	0	3.6	0	17.1	0	17.1	8.1

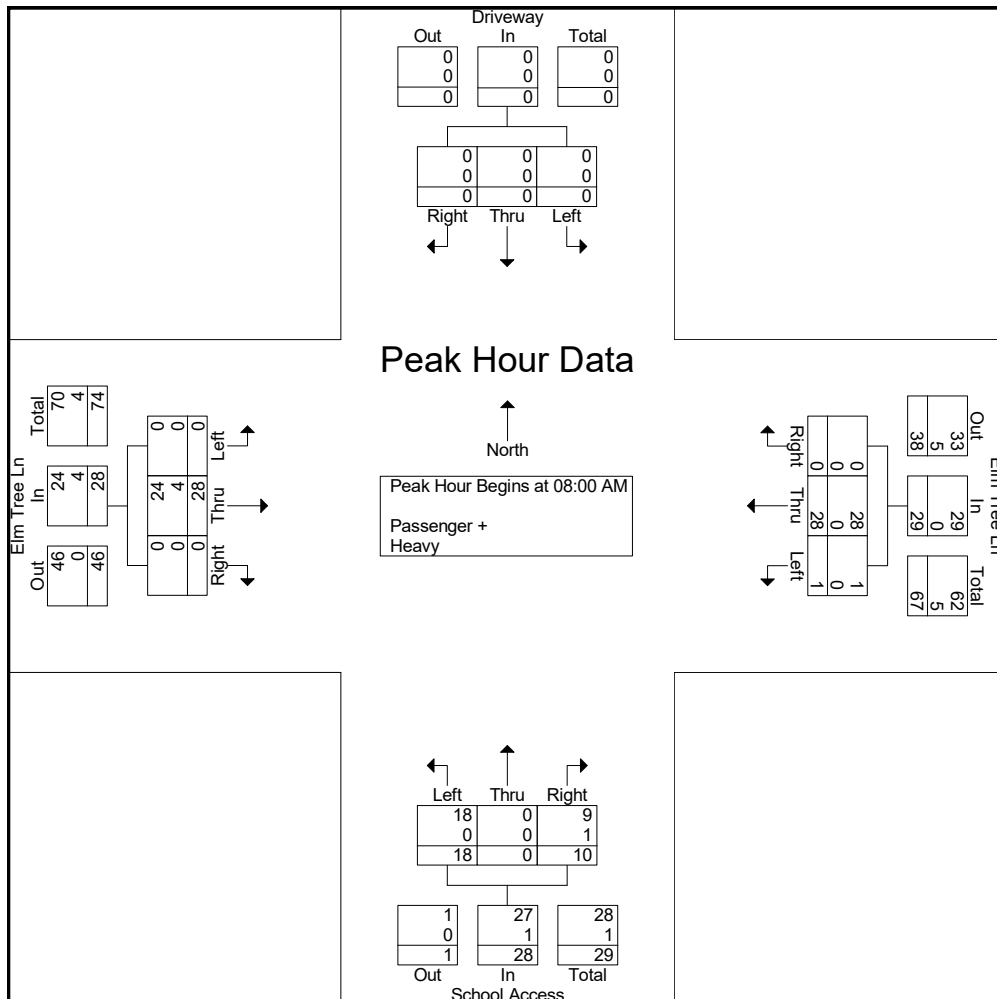
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623c  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Driveway Southbound				Elm Tree Ln Westbound				School Access Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 08:00 AM																	
08:00 AM	0	0	0	0	0	2	0	2	0	0	0	0	0	3	0	3	5
08:15 AM	0	0	0	0	0	4	1	5	1	0	2	3	0	7	0	7	15
08:30 AM	0	0	0	0	0	12	0	12	8	0	13	21	0	11	0	11	44
08:45 AM	0	0	0	0	0	10	0	10	1	0	3	4	0	7	0	7	21
Total Volume	0	0	0	0	0	28	1	29	10	0	18	28	0	28	0	28	85
% App. Total	0	0	0	0	0	96.6	3.4		35.7	0	64.3		0	100	0		
PHF	.000	.000	.000	.000	.000	.583	.250	.604	.313	.000	.346	.333	.000	.636	.000	.636	.483
Passenger +	0	0	0	0	0	28	1	29	9	0	18	27	0	24	0	24	80
% Passenger +	0	0	0	0	0	100	100	100	90.0	0	100	96.4	0	85.7	0	85.7	94.1
Heavy	0	0	0	0	0	0	0	0	1	0	0	1	0	4	0	4	5
% Heavy	0	0	0	0	0	0	0	0	10.0	0	0	3.6	0	14.3	0	14.3	5.9



# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623g  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Driveway Southbound				Elm Tree Ln Westbound				Annapolis St Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
07:00 AM	0	0	0	0	0	2	3	5	1	0	0	1	0	1	0	1	7
07:15 AM	0	0	1	1	0	1	0	1	2	0	1	3	0	1	1	2	7
07:30 AM	0	0	0	0	0	2	0	2	3	0	0	3	1	1	0	2	7
07:45 AM	0	0	0	0	0	2	0	2	1	0	0	1	0	4	0	4	7
<b>Total</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>7</b>	<b>3</b>	<b>10</b>	<b>7</b>	<b>0</b>	<b>1</b>	<b>8</b>	<b>1</b>	<b>7</b>	<b>1</b>	<b>9</b>	<b>28</b>
08:00 AM	0	0	0	0	0	3	0	3	1	0	0	1	0	4	0	4	8
08:15 AM	1	0	0	1	0	6	0	6	7	0	0	7	0	3	1	4	18
08:30 AM	0	0	0	0	0	8	0	8	45	0	2	47	0	19	1	20	75
08:45 AM	0	0	0	0	0	8	1	9	0	0	0	0	0	8	0	8	17
<b>Total</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>25</b>	<b>1</b>	<b>26</b>	<b>53</b>	<b>0</b>	<b>2</b>	<b>55</b>	<b>0</b>	<b>34</b>	<b>2</b>	<b>36</b>	<b>118</b>
<b>Grand Total</b>	<b>1</b>	<b>0</b>	<b>1</b>	<b>2</b>	<b>0</b>	<b>32</b>	<b>4</b>	<b>36</b>	<b>60</b>	<b>0</b>	<b>3</b>	<b>63</b>	<b>1</b>	<b>41</b>	<b>3</b>	<b>45</b>	<b>146</b>
Apprch %	50	0	50		0	88.9	11.1		95.2	0	4.8		2.2	91.1	6.7		
Total %	0.7	0	0.7	1.4	0	21.9	2.7	24.7	41.1	0	2.1	43.2	0.7	28.1	2.1	30.8	
Passenger +	1	0	1	2	0	31	4	35	60	0	3	63	1	33	3	37	137
% Passenger +	100	0	100	100	0	96.9	100	97.2	100	0	100	100	100	80.5	100	82.2	93.8
Heavy	0	0	0	0	0	1	0	1	0	0	0	0	0	8	0	8	9
% Heavy	0	0	0	0	0	3.1	0	2.8	0	0	0	0	0	19.5	0	17.8	6.2

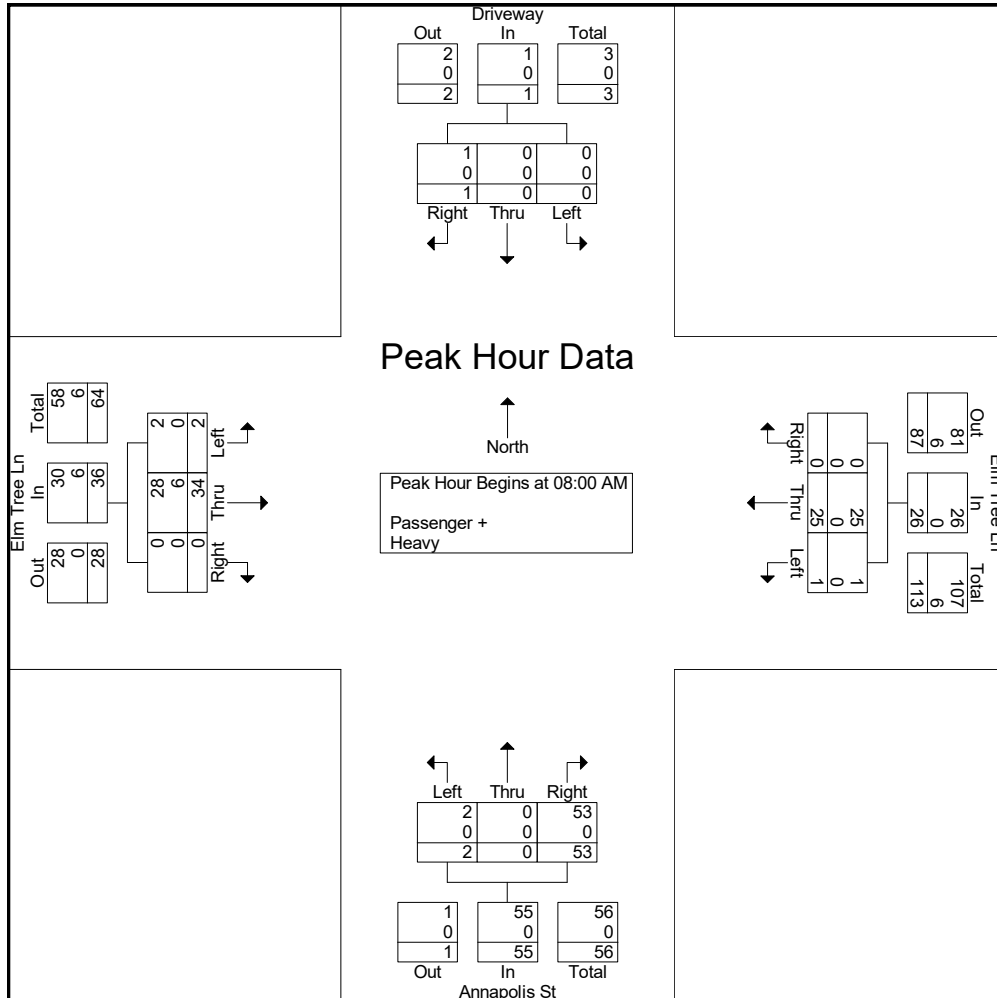
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623g  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Driveway Southbound				Elm Tree Ln Westbound				Annapolis St Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 08:00 AM																	
08:00 AM	0	0	0	0	0	3	0	3	1	0	0	1	0	4	0	4	8
08:15 AM	1	0	0	1	0	6	0	6	7	0	0	7	0	3	1	4	18
08:30 AM	0	0	0	0	0	8	0	8	<b>45</b>	0	<b>2</b>	<b>47</b>	0	<b>19</b>	1	<b>20</b>	<b>75</b>
08:45 AM	0	0	0	0	0	8	1	9	0	0	0	0	0	8	0	8	17
Total Volume	1	0	0	1	0	25	1	26	53	0	2	55	0	34	2	36	118
% App. Total	100	0	0		0	96.2	3.8		96.4	0	3.6		0	94.4	5.6		
PHF	.250	.000	.000	.250	.000	.781	.250	.722	.294	.000	.250	.293	.000	.447	.500	.450	.393
Passenger +	1	0	0	1	0	25	1	26	53	0	2	55	0	28	2	30	112
% Passenger +	100	0	0	100	0	100	100	100	100	0	100	100	0	82.4	100	83.3	94.9
Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	6	0	6	6
% Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	17.6	0	16.7	5.1



# Exhibit E

## Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623i  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Alameda Ave Southbound			Annapolis St Westbound			Alameda Ave Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
07:00 AM	16	2	18	0	0	0	0	24	24	42
07:15 AM	12	1	13	2	0	2	1	44	45	60
07:30 AM	24	1	25	1	1	2	2	50	52	79
07:45 AM	36	0	36	1	0	1	1	59	60	97
Total	88	4	92	4	1	5	4	177	181	278
08:00 AM	25	0	25	4	0	4	0	65	65	94
08:15 AM	25	12	37	1	1	2	7	60	67	106
08:30 AM	57	15	72	1	0	1	14	71	85	158
08:45 AM	39	0	39	1	0	1	0	37	37	77
Total	146	27	173	7	1	8	21	233	254	435
Grand Total	234	31	265	11	2	13	25	410	435	713
Apprch %	88.3	11.7		84.6	15.4		5.7	94.3		
Total %	32.8	4.3	37.2	1.5	0.3	1.8	3.5	57.5	61	
Passenger +	232	31	263	11	2	13	25	401	426	702
% Passenger +	99.1	100	99.2	100	100	100	100	97.8	97.9	98.5
Heavy	2	0	2	0	0	0	0	9	9	11
% Heavy	0.9	0	0.8	0	0	0	0	2.2	2.1	1.5

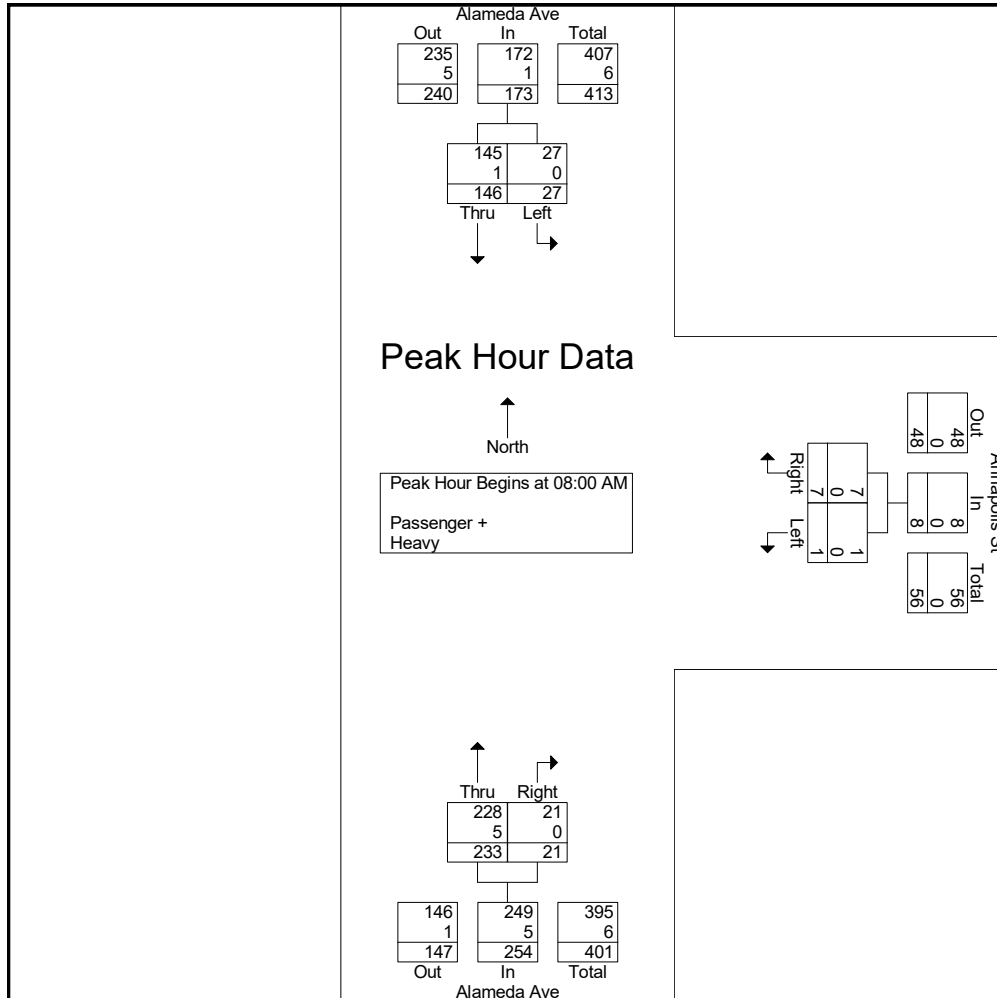
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623i  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Alameda Ave Southbound			Annapolis St Westbound			Alameda Ave Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 08:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 08:00 AM										
08:00 AM	25	0	25	4	0	4	0	65	65	94
08:15 AM	25	12	37	1	1	2	7	60	67	106
08:30 AM	<b>57</b>	<b>15</b>	<b>72</b>	1	0	1	<b>14</b>	<b>71</b>	<b>85</b>	<b>158</b>
08:45 AM	39	0	39	1	0	1	0	37	37	77
Total Volume	146	27	173	7	1	8	21	233	254	435
% App. Total	84.4	15.6		87.5	12.5		8.3	91.7		
PHF	.640	.450	.601	.438	.250	.500	.375	.820	.747	.688
Passenger +	145	27	172	7	1	8	21	228	249	429
% Passenger +	99.3	100	99.4	100	100	100	100	97.9	98.0	98.6
Heavy	1	0	1	0	0	0	0	5	5	6
% Heavy	0.7	0	0.6	0	0	0	0	2.1	2.0	1.4



# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623e  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Alameda Ave Southbound				School Access Westbound				Alameda Ave Northbound				Exclu. Total	Inclu. Total	Int. Total
	Thru	Left	Peds	App. Total	Right	Left	Peds	App. Total	Right	Thru	Peds	App. Total			
07:00 AM	18	3	0	21	0	0	0	0	1	23	0	24	0	45	45
07:15 AM	13	0	0	13	0	0	1	0	0	45	1	45	2	58	60
07:30 AM	26	2	0	28	0	0	0	0	0	50	0	50	0	78	78
07:45 AM	36	5	0	41	0	0	0	0	5	56	0	61	0	102	102
<b>Total</b>	<b>93</b>	<b>10</b>	<b>0</b>	<b>103</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>6</b>	<b>174</b>	<b>1</b>	<b>180</b>	<b>2</b>	<b>283</b>	<b>285</b>
08:00 AM	23	4	0	27	0	0	1	0	3	64	1	67	2	94	96
08:15 AM	39	7	0	46	0	0	0	0	7	53	0	60	0	106	106
08:30 AM	65	8	0	73	2	9	0	11	19	51	0	70	0	154	154
08:45 AM	36	1	0	37	1	3	0	4	2	37	0	39	0	80	80
<b>Total</b>	<b>163</b>	<b>20</b>	<b>0</b>	<b>183</b>	<b>3</b>	<b>12</b>	<b>1</b>	<b>15</b>	<b>31</b>	<b>205</b>	<b>1</b>	<b>236</b>	<b>2</b>	<b>434</b>	<b>436</b>
<b>Grand Total</b>	<b>256</b>	<b>30</b>	<b>0</b>	<b>286</b>	<b>3</b>	<b>12</b>	<b>2</b>	<b>15</b>	<b>37</b>	<b>379</b>	<b>2</b>	<b>416</b>	<b>4</b>	<b>717</b>	<b>721</b>
Apprch %	89.5	10.5			20	80			8.9	91.1					
Total %	35.7	4.2		39.9	0.4	1.7		2.1	5.2	52.9		58	0.6	99.4	
Passenger +	256	30		286	3	12		17	37	379		418	0	0	721
% Passenger +	100	100	0	100	100	100	100	100	100	100	100	100	0	0	100
Heavy	0	0		0	0	0		0	0	0		0	0	0	0
% Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

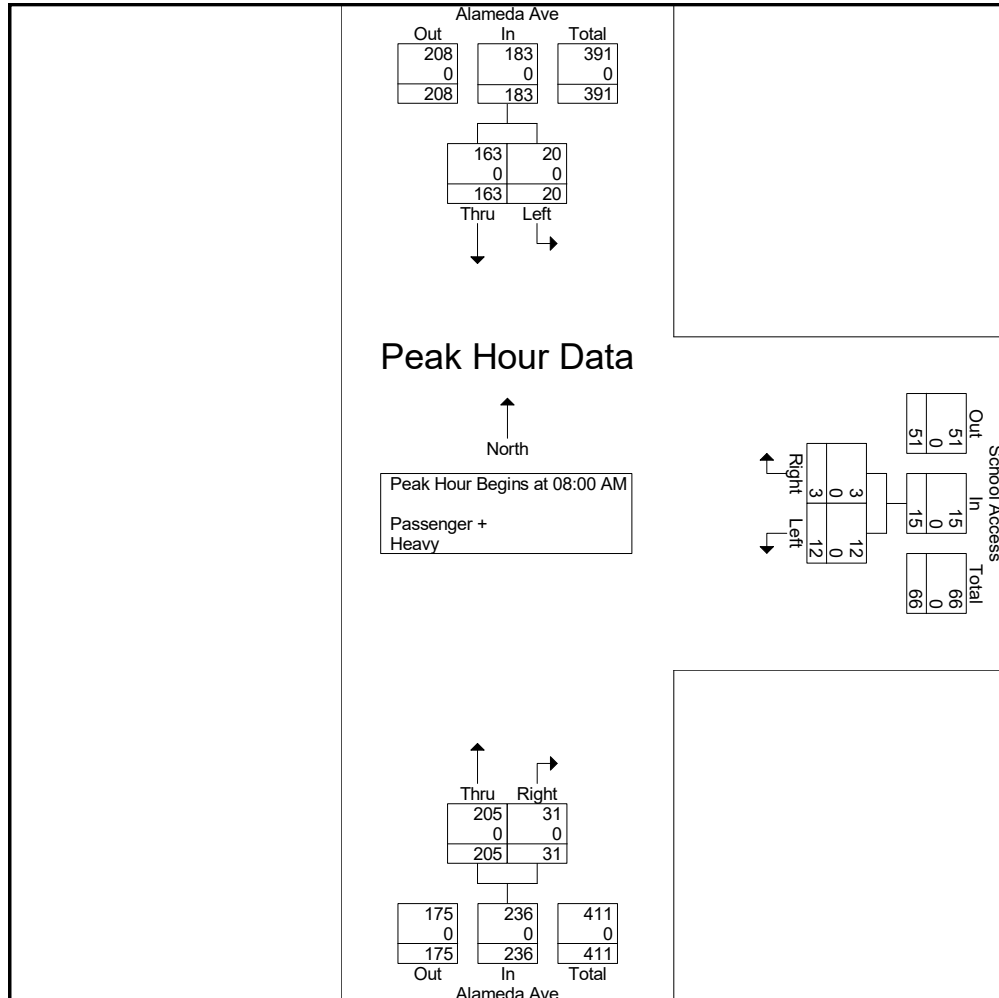
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623e  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Alameda Ave Southbound			School Access Westbound			Alameda Ave Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 08:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 08:00 AM										
08:00 AM	23	4	27	0	0	0	3	64	67	94
08:15 AM	39	7	46	0	0	0	7	53	60	106
08:30 AM	<b>65</b>	<b>8</b>	<b>73</b>	<b>2</b>	<b>9</b>	<b>11</b>	<b>19</b>	<b>51</b>	<b>70</b>	<b>154</b>
08:45 AM	36	1	37	1	3	4	2	37	39	80
Total Volume	163	20	183	3	12	15	31	205	236	434
% App. Total	89.1	10.9		20	80		13.1	86.9		
PHF	.627	.625	.627	.375	.333	.341	.408	.801	.843	.705
Passenger +	163	20	183	3	12	15	31	205	236	434
% Passenger +	100	100	100	100	100	100	100	100	100	100
Heavy	0	0	0	0	0	0	0	0	0	0
% Heavy	0	0	0	0	0	0	0	0	0	0



# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623b  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Alameda Ave Southbound				Elm Tree Ln Westbound				Alameda Ave Northbound				Driveway Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
02:00 PM	0	28	0	28	1	0	7	8	1	25	0	26	0	0	0	0	62
02:15 PM	0	49	4	53	0	0	2	2	0	39	0	39	0	0	0	0	94
02:30 PM	0	42	1	43	1	0	4	5	1	39	0	40	0	0	0	0	88
02:45 PM	0	61	5	66	3	0	4	7	6	46	0	52	0	0	0	0	125
<b>Total</b>	<b>0</b>	<b>180</b>	<b>10</b>	<b>190</b>	<b>5</b>	<b>0</b>	<b>17</b>	<b>22</b>	<b>8</b>	<b>149</b>	<b>0</b>	<b>157</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>369</b>
03:00 PM	0	82	2	84	4	0	8	12	4	46	0	50	0	0	0	0	146
03:15 PM	0	63	2	65	6	0	12	18	4	45	0	49	0	0	0	0	132
03:30 PM	0	55	1	56	3	0	4	7	4	31	0	35	0	0	1	1	99
03:45 PM	1	60	6	67	4	0	5	9	1	39	0	40	0	0	0	0	116
<b>Total</b>	<b>1</b>	<b>260</b>	<b>11</b>	<b>272</b>	<b>17</b>	<b>0</b>	<b>29</b>	<b>46</b>	<b>13</b>	<b>161</b>	<b>0</b>	<b>174</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>493</b>
04:00 PM	0	57	2	59	1	0	1	2	2	45	0	47	0	0	0	0	108
04:15 PM	0	61	1	62	1	0	2	3	0	51	0	51	0	0	1	1	117
04:30 PM	0	56	3	59	1	0	3	4	4	58	0	62	0	0	0	0	125
04:45 PM	0	64	1	65	1	0	4	5	5	38	0	43	0	0	0	0	113
<b>Total</b>	<b>0</b>	<b>238</b>	<b>7</b>	<b>245</b>	<b>4</b>	<b>0</b>	<b>10</b>	<b>14</b>	<b>11</b>	<b>192</b>	<b>0</b>	<b>203</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>463</b>
05:00 PM	1	75	2	78	0	0	5	5	5	44	0	49	0	0	1	1	133
05:15 PM	0	58	5	63	0	0	8	8	2	43	0	45	0	0	0	0	116
05:30 PM	0	54	3	57	3	0	7	10	6	51	0	57	0	0	0	0	124
05:45 PM	0	56	3	59	3	0	4	7	5	54	0	59	0	0	0	0	125
<b>Total</b>	<b>1</b>	<b>243</b>	<b>13</b>	<b>257</b>	<b>6</b>	<b>0</b>	<b>24</b>	<b>30</b>	<b>18</b>	<b>192</b>	<b>0</b>	<b>210</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>498</b>
<b>Grand Total</b>	<b>2</b>	<b>921</b>	<b>41</b>	<b>964</b>	<b>32</b>	<b>0</b>	<b>80</b>	<b>112</b>	<b>50</b>	<b>694</b>	<b>0</b>	<b>744</b>	<b>0</b>	<b>0</b>	<b>3</b>	<b>3</b>	<b>1823</b>
<b>Apprch %</b>	<b>0.2</b>	<b>95.5</b>	<b>4.3</b>		<b>28.6</b>	<b>0</b>	<b>71.4</b>		<b>6.7</b>	<b>93.3</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>100</b>		
<b>Total %</b>	<b>0.1</b>	<b>50.5</b>	<b>2.2</b>	<b>52.9</b>	<b>1.8</b>	<b>0</b>	<b>4.4</b>	<b>6.1</b>	<b>2.7</b>	<b>38.1</b>	<b>0</b>	<b>40.8</b>	<b>0</b>	<b>0</b>	<b>0.2</b>	<b>0.2</b>	
<b>Passenger +</b>	<b>2</b>	<b>906</b>	<b>36</b>	<b>944</b>	<b>32</b>	<b>0</b>	<b>77</b>	<b>109</b>	<b>47</b>	<b>690</b>	<b>0</b>	<b>737</b>	<b>0</b>	<b>0</b>	<b>3</b>	<b>3</b>	<b>1793</b>
<b>% Passenger +</b>	<b>100</b>	<b>98.4</b>	<b>87.8</b>	<b>97.9</b>	<b>100</b>	<b>0</b>	<b>96.2</b>	<b>97.3</b>	<b>94</b>	<b>99.4</b>	<b>0</b>	<b>99.1</b>	<b>0</b>	<b>0</b>	<b>100</b>	<b>100</b>	<b>98.4</b>
<b>Heavy</b>	<b>0</b>	<b>15</b>	<b>5</b>	<b>20</b>	<b>0</b>	<b>0</b>	<b>3</b>	<b>3</b>	<b>3</b>	<b>4</b>	<b>0</b>	<b>7</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>30</b>
<b>% Heavy</b>	<b>0</b>	<b>1.6</b>	<b>12.2</b>	<b>2.1</b>	<b>0</b>	<b>0</b>	<b>3.8</b>	<b>2.7</b>	<b>6</b>	<b>0.6</b>	<b>0</b>	<b>0.9</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1.6</b>

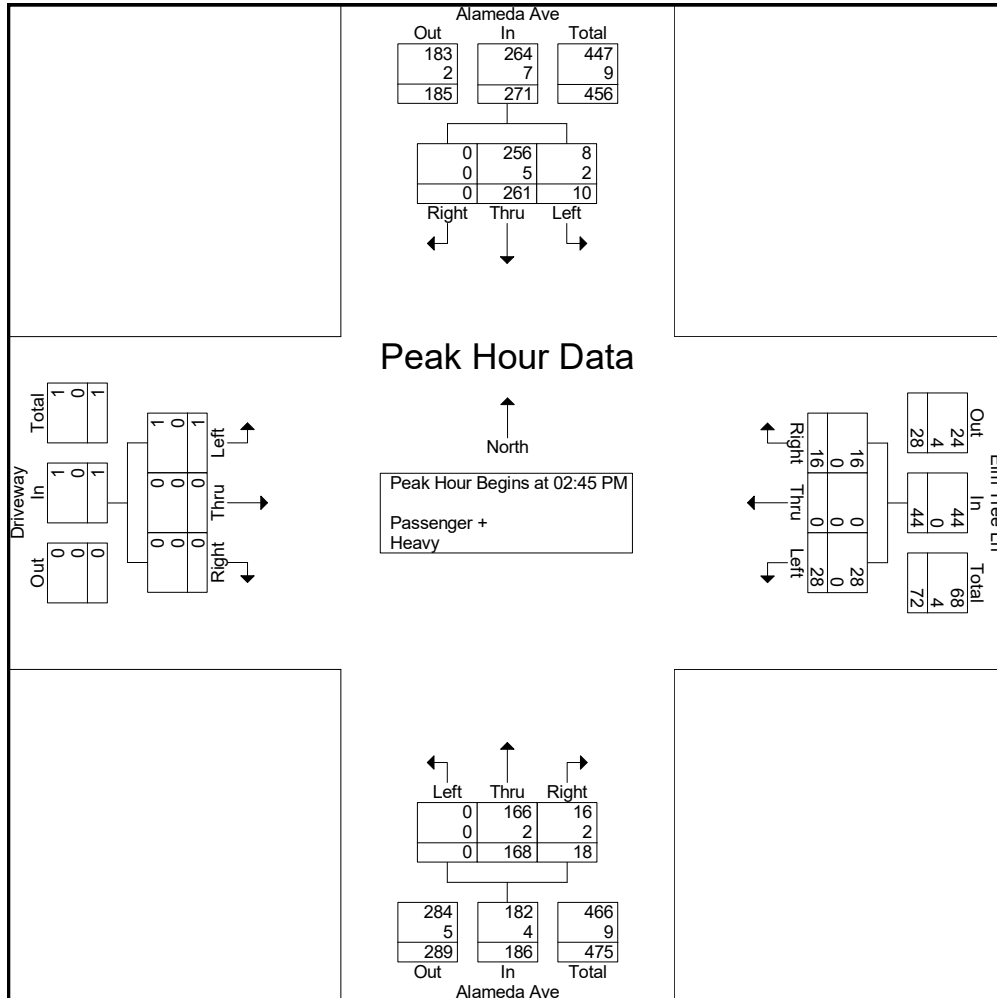
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623b  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Alameda Ave Southbound				Elm Tree Ln Westbound				Alameda Ave Northbound				Driveway Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 02:00 PM to 03:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 02:45 PM																	
02:45 PM	0	61	5	66	3	0	4	7	6	46	0	52	0	0	0	0	125
03:00 PM	0	82	2	84	4	0	8	12	4	46	0	50	0	0	0	0	146
03:15 PM	0	63	2	65	6	0	12	18	4	45	0	49	0	0	0	0	132
03:30 PM	0	55	1	56	3	0	4	7	4	31	0	35	0	0	1	1	99
Total Volume	0	261	10	271	16	0	28	44	18	168	0	186	0	0	1	1	502
% App. Total	0	96.3	3.7		36.4	0	63.6		9.7	90.3	0		0	0	100		
PHF	.000	.796	.500	.807	.667	.000	.583	.611	.750	.913	.000	.894	.000	.000	.250	.250	.860
Passenger +	0	256	8	264	16	0	28	44	16	166	0	182	0	0	1	1	491
% Passenger +	0	98.1	80.0	97.4	100	0	100	100	88.9	98.8	0	97.8	0	0	100	100	97.8
Heavy	0	5	2	7	0	0	0	0	2	2	0	4	0	0	0	0	11
% Heavy	0	1.9	20.0	2.6	0	0	0	0	11.1	1.2	0	2.2	0	0	0	0	2.2



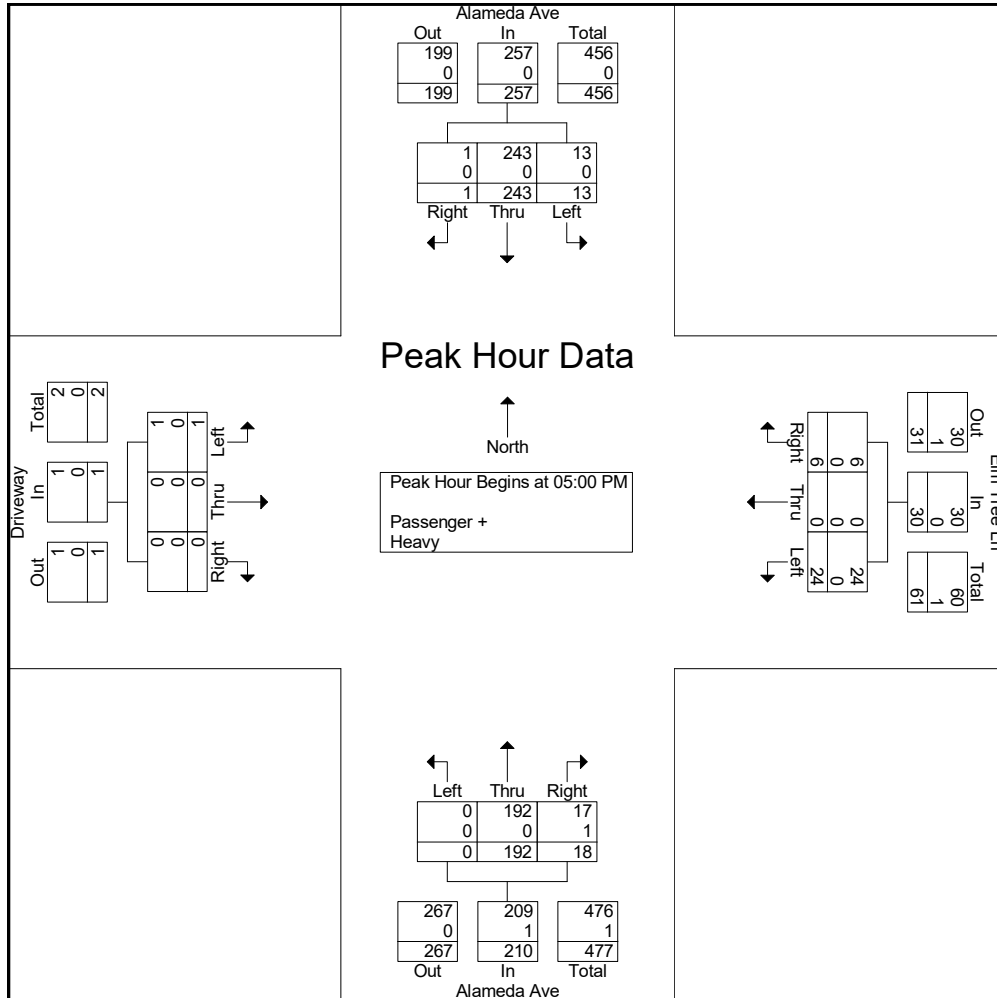
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623b  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 3

Start Time	Alameda Ave Southbound				Elm Tree Ln Westbound				Alameda Ave Northbound				Driveway Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	1	75	2	78	0	0	5	5	5	44	0	49	0	0	1	1	133
05:15 PM	0	58	5	63	0	0	8	8	2	43	0	45	0	0	0	0	116
05:30 PM	0	54	3	57	3	0	7	10	6	51	0	57	0	0	0	0	124
05:45 PM	0	56	3	59	3	0	4	7	5	54	0	59	0	0	0	0	125
Total Volume	1	243	13	257	6	0	24	30	18	192	0	210	0	0	1	1	498
% App. Total	0.4	94.6	5.1		20	0	80		8.6	91.4	0		0	0	100		
PHF	.250	.810	.650	.824	.500	.000	.750	.750	.750	.889	.000	.890	.000	.000	.250	.250	.936
Passenger +	1	243	13	257	6	0	24	30	17	192	0	209	0	0	1	1	497
% Passenger +	100	100	100	100	100	0	100	100	94.4	100	0	99.5	0	0	100	100	99.8
Heavy	0	0	0	0	0	0	0	0	1	0	0	1	0	0	0	0	1
% Heavy	0	0	0	0	0	0	0	0	5.6	0	0	0.5	0	0	0	0	0.2



# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623d  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Driveway Southbound				Elm Tree Ln Westbound				School Access Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
02:00 PM	0	0	0	0	0	8	0	8	1	0	0	1	0	1	0	1	10
02:15 PM	0	0	0	0	0	2	0	2	0	0	0	0	0	4	0	4	6
02:30 PM	0	0	0	0	0	4	1	5	1	0	1	2	0	2	0	2	9
02:45 PM	0	0	0	0	0	5	0	5	0	0	2	2	0	11	0	11	18
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>19</b>	<b>1</b>	<b>20</b>	<b>2</b>	<b>0</b>	<b>3</b>	<b>5</b>	<b>0</b>	<b>18</b>	<b>0</b>	<b>18</b>	<b>43</b>
03:00 PM	0	0	0	0	0	9	2	11	4	0	3	7	1	6	0	7	25
03:15 PM	0	0	0	0	0	15	0	15	4	0	2	6	0	5	0	5	26
03:30 PM	0	0	0	0	0	4	0	4	2	0	4	6	0	5	0	5	15
03:45 PM	0	0	0	0	0	4	1	5	1	0	4	5	0	5	0	5	15
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>32</b>	<b>3</b>	<b>35</b>	<b>11</b>	<b>0</b>	<b>13</b>	<b>24</b>	<b>1</b>	<b>21</b>	<b>0</b>	<b>22</b>	<b>81</b>
04:00 PM	0	0	0	0	0	1	0	1	1	0	1	2	0	3	0	3	6
04:15 PM	0	0	0	0	1	4	0	5	3	0	1	4	0	1	0	1	10
04:30 PM	0	0	0	0	0	3	0	3	0	0	0	0	0	6	0	6	9
04:45 PM	0	0	0	0	0	5	0	5	1	0	0	1	0	5	0	5	11
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>13</b>	<b>0</b>	<b>14</b>	<b>5</b>	<b>0</b>	<b>2</b>	<b>7</b>	<b>0</b>	<b>15</b>	<b>0</b>	<b>15</b>	<b>36</b>
05:00 PM	0	0	0	0	0	5	1	6	0	0	0	0	0	7	0	7	13
05:15 PM	0	0	0	0	0	8	0	8	0	0	0	0	0	7	0	7	15
05:30 PM	0	0	0	0	0	7	0	7	2	0	1	3	0	9	0	9	19
05:45 PM	0	0	0	0	0	6	4	10	3	0	1	4	1	5	0	6	20
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>26</b>	<b>5</b>	<b>31</b>	<b>5</b>	<b>0</b>	<b>2</b>	<b>7</b>	<b>1</b>	<b>28</b>	<b>0</b>	<b>29</b>	<b>67</b>
<b>Grand Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>90</b>	<b>9</b>	<b>100</b>	<b>23</b>	<b>0</b>	<b>20</b>	<b>43</b>	<b>2</b>	<b>82</b>	<b>0</b>	<b>84</b>	<b>227</b>
Apprch %	0	0	0	0	1	90	9	100	53.5	0	46.5	100	2.4	97.6	0	100	
Total %	0	0	0	0	0.4	39.6	4	44.1	10.1	0	8.8	18.9	0.9	36.1	0	37	
Passenger +	0	0	0	0	1	88	9	98	23	0	20	43	2	74	0	76	217
% Passenger +	0	0	0	0	100	97.8	100	98	100	0	100	100	100	90.2	0	90.5	95.6
Heavy	0	0	0	0	0	2	0	2	0	0	0	0	0	8	0	8	10
% Heavy	0	0	0	0	0	2.2	0	2	0	0	0	0	0	9.8	0	9.5	4.4

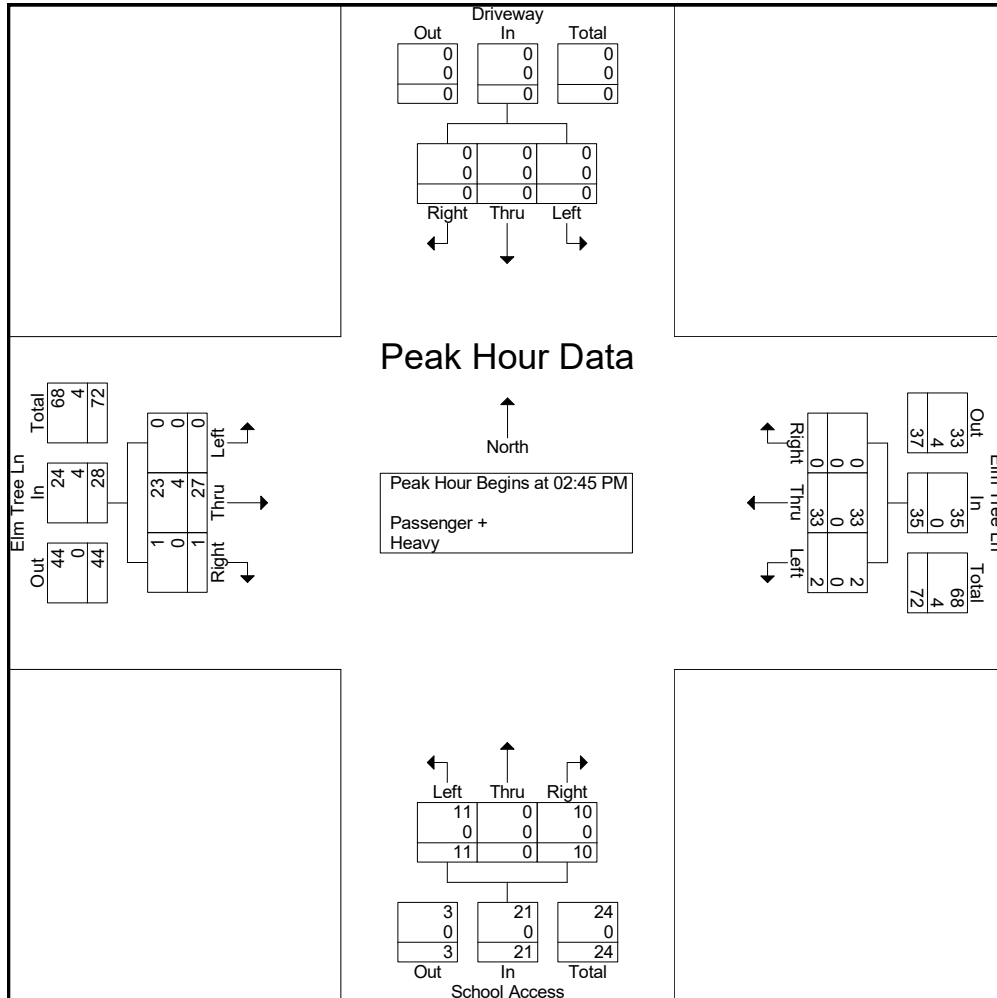
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623d  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Driveway Southbound				Elm Tree Ln Westbound				School Access Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 02:00 PM to 03:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 02:45 PM																	
02:45 PM	0	0	0	0	0	5	0	5	0	0	2	2	0	11	0	11	18
03:00 PM	0	0	0	0	0	9	2	11	4	0	3	7	1	6	0	7	25
03:15 PM	0	0	0	0	0	15	0	15	4	0	2	6	0	5	0	5	26
03:30 PM	0	0	0	0	0	4	0	4	2	0	4	6	0	5	0	5	15
Total Volume	0	0	0	0	0	33	2	35	10	0	11	21	1	27	0	28	84
% App. Total	0	0	0		0	94.3	5.7		47.6	0	52.4		3.6	96.4	0		
PHF	.000	.000	.000	.000	.000	.550	.250	.583	.625	.000	.688	.750	.250	.614	.000	.636	.808
Passenger +	0	0	0	0	0	33	2	35	10	0	11	21	1	23	0	24	80
% Passenger +	0	0	0		0	100	100	100	100	0	100	100	100	85.2	0	85.7	95.2
Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	4	0	4	4
% Heavy	0	0	0		0	0	0		0	0	0		0	14.8	0	14.3	4.8



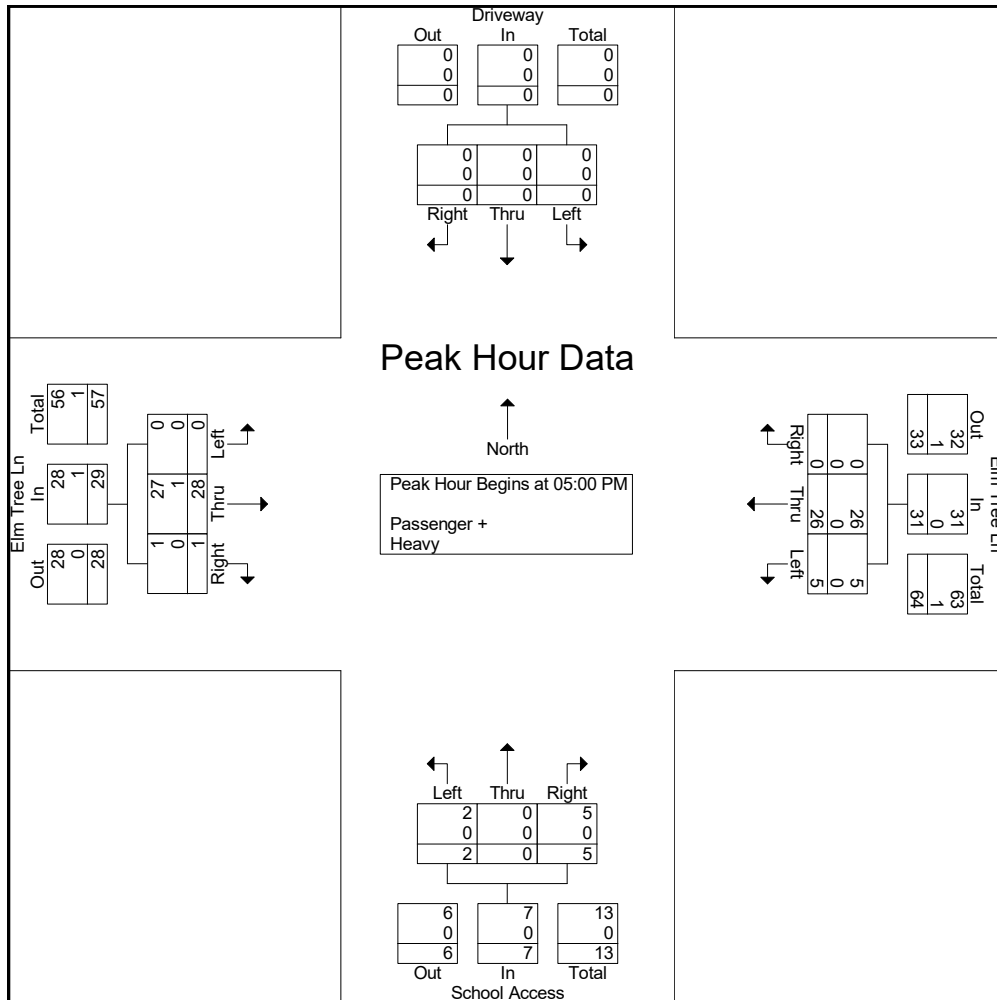
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623d  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 3

Start Time	Driveway Southbound				Elm Tree Ln Westbound				School Access Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	0	0	0	0	0	5	1	6	0	0	0	0	0	7	0	7	13
05:15 PM	0	0	0	0	0	8	0	8	0	0	0	0	0	7	0	7	15
05:30 PM	0	0	0	0	0	7	0	7	2	0	1	3	0	9	0	9	19
05:45 PM	0	0	0	0	0	6	4	10	3	0	1	4	1	5	0	6	20
Total Volume	0	0	0	0	0	26	5	31	5	0	2	7	1	28	0	29	67
% App. Total	0	0	0	0	0	83.9	16.1		71.4	0	28.6		3.4	96.6	0		
PHF	.000	.000	.000	.000	.000	.813	.313	.775	.417	.000	.500	.438	.250	.778	.000	.806	.838
Passenger +	0	0	0	0	0	26	5	31	5	0	2	7	1	27	0	28	66
% Passenger +	0	0	0	0	0	100	100	100	100	0	100	100	100	96.4	0	96.6	98.5
Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1	1
% Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	3.6	0	3.4	1.5



# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623h  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Driveway Southbound				Elm Tree Ln Westbound				Annapolis St Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
02:00 PM	0	0	0	0	0	7	0	7	0	0	0	0	0	2	0	2	9
02:15 PM	0	0	0	0	0	3	0	3	0	0	0	0	0	3	0	3	6
02:30 PM	0	0	1	1	0	6	0	6	0	0	0	0	0	2	1	3	10
02:45 PM	0	0	0	0	0	6	0	6	0	0	0	0	0	8	0	8	14
<b>Total</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>22</b>	<b>0</b>	<b>22</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>15</b>	<b>1</b>	<b>16</b>	<b>39</b>
03:00 PM	0	0	0	0	0	7	0	7	16	0	0	16	0	5	0	5	28
03:15 PM	0	0	0	0	0	8	0	8	23	0	6	29	0	11	0	11	48
03:30 PM	0	0	0	0	0	5	2	7	2	0	0	2	1	6	0	7	16
03:45 PM	0	0	0	0	0	5	0	5	2	0	0	2	1	4	0	5	12
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>25</b>	<b>2</b>	<b>27</b>	<b>43</b>	<b>0</b>	<b>6</b>	<b>49</b>	<b>2</b>	<b>26</b>	<b>0</b>	<b>28</b>	<b>104</b>
04:00 PM	0	0	0	0	0	1	2	3	0	0	0	0	1	4	0	5	8
04:15 PM	0	0	0	0	0	4	0	4	1	0	1	2	2	2	1	5	11
04:30 PM	0	0	0	0	0	0	2	2	1	0	1	2	0	5	0	5	9
04:45 PM	0	0	0	0	0	5	1	6	2	0	1	3	0	7	0	7	16
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>10</b>	<b>5</b>	<b>15</b>	<b>4</b>	<b>0</b>	<b>3</b>	<b>7</b>	<b>3</b>	<b>18</b>	<b>1</b>	<b>22</b>	<b>44</b>
05:00 PM	0	0	0	0	0	4	0	4	2	0	0	2	0	5	0	5	11
05:15 PM	0	0	0	0	0	8	0	8	1	0	1	2	0	6	1	7	17
05:30 PM	0	0	0	0	0	9	2	11	1	0	1	2	1	11	0	12	25
05:45 PM	0	0	0	0	0	7	1	8	2	0	1	3	1	6	0	7	18
<b>Total</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>28</b>	<b>3</b>	<b>31</b>	<b>6</b>	<b>0</b>	<b>3</b>	<b>9</b>	<b>2</b>	<b>28</b>	<b>1</b>	<b>31</b>	<b>71</b>
<b>Grand Total</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>85</b>	<b>10</b>	<b>95</b>	<b>53</b>	<b>0</b>	<b>12</b>	<b>65</b>	<b>7</b>	<b>87</b>	<b>3</b>	<b>97</b>	<b>258</b>
Apprch %	0	0	100		0	89.5	10.5		81.5	0	18.5		7.2	89.7	3.1		
Total %	0	0	0.4	0.4	0	32.9	3.9	36.8	20.5	0	4.7	25.2	2.7	33.7	1.2	37.6	
Passenger +	0	0	1	1	0	81	10	91	53	0	12	65	7	78	3	88	245
% Passenger +	0	0	100	100	0	95.3	100	95.8	100	0	100	100	100	89.7	100	90.7	95
Heavy	0	0	0	0	0	4	0	4	0	0	0	0	0	9	0	9	13
% Heavy	0	0	0	0	0	4.7	0	4.2	0	0	0	0	0	10.3	0	9.3	5

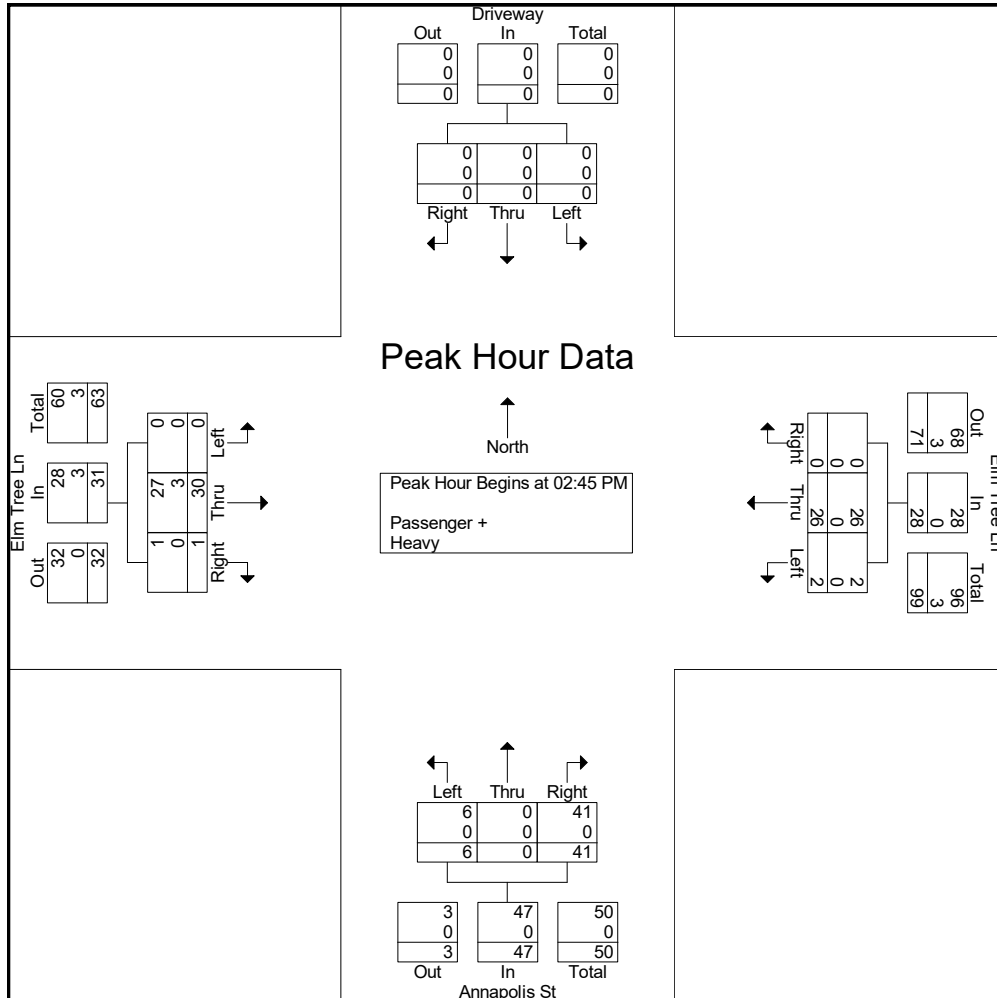
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623h  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Driveway Southbound				Elm Tree Ln Westbound				Annapolis St Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 02:00 PM to 03:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 02:45 PM																	
02:45 PM	0	0	0	0	0	6	0	6	0	0	0	0	0	8	0	8	14
03:00 PM	0	0	0	0	0	7	0	7	16	0	0	16	0	5	0	5	28
03:15 PM	0	0	0	0	0	<b>8</b>	0	<b>8</b>	<b>23</b>	0	<b>6</b>	<b>29</b>	0	<b>11</b>	0	<b>11</b>	<b>48</b>
03:30 PM	0	0	0	0	0	5	<b>2</b>	7	2	0	0	2	<b>1</b>	6	0	7	16
Total Volume	0	0	0	0	0	26	2	28	41	0	6	47	1	30	0	31	106
% App. Total	0	0	0	0	0	92.9	7.1		87.2	0	12.8		3.2	96.8	0		
PHF	.000	.000	.000	.000	.000	.813	.250	.875	.446	.000	.250	.405	.250	.682	.000	.705	.552
Passenger +	0	0	0	0	0	26	2	28	41	0	6	47	1	27	0	28	103
% Passenger +	0	0	0	0	0	100	100	100	100	0	100	100	100	90.0	0	90.3	97.2
Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	3	3
% Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	10.0	0	9.7	2.8



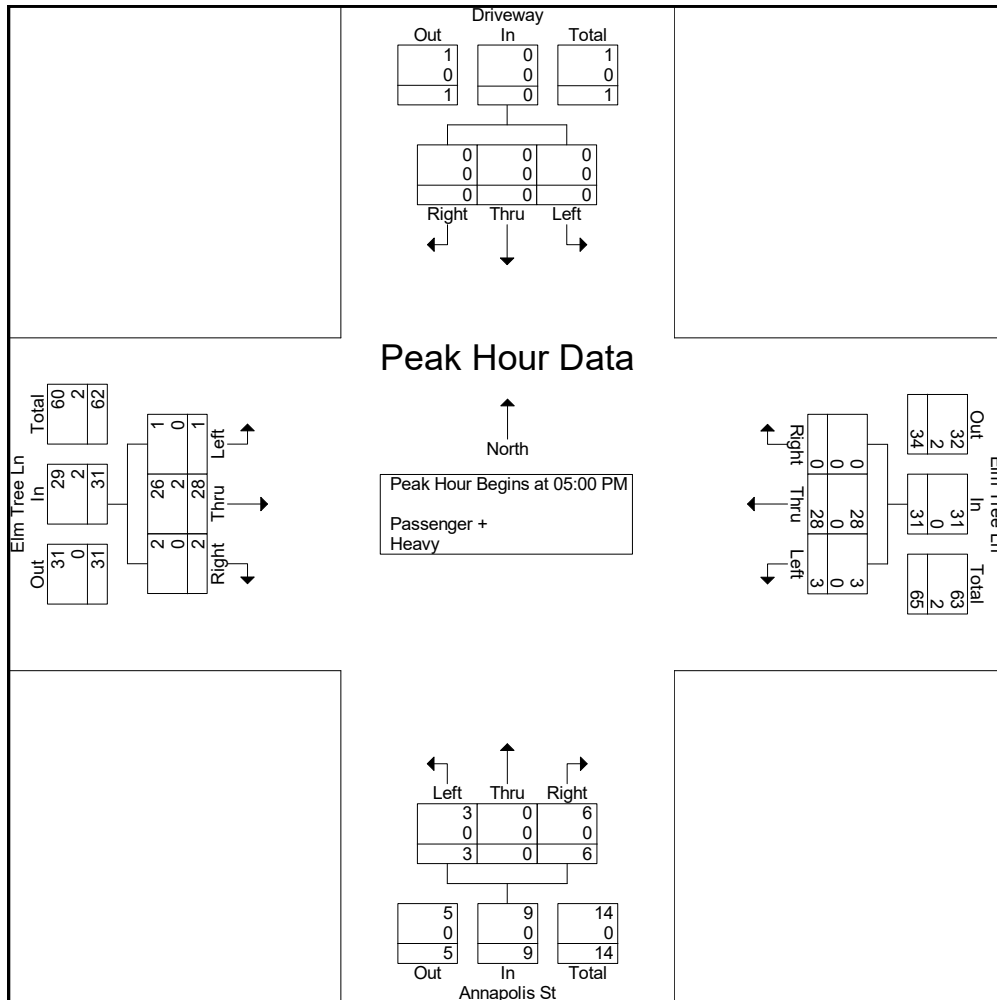
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623h  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 3

Start Time	Driveway Southbound				Elm Tree Ln Westbound				Annapolis St Northbound				Elm Tree Ln Eastbound				Int. Total
	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 05:00 PM																	
05:00 PM	0	0	0	0	0	4	0	4	2	0	0	2	0	5	0	5	11
05:15 PM	0	0	0	0	0	8	0	8	1	0	1	2	0	6	1	7	17
05:30 PM	0	0	0	0	0	<b>9</b>	<b>2</b>	<b>11</b>	1	0	1	2	<b>1</b>	<b>11</b>	0	<b>12</b>	<b>25</b>
05:45 PM	0	0	0	0	0	7	1	8	2	0	1	3	1	6	0	7	18
Total Volume	0	0	0	0	0	28	3	31	6	0	3	9	2	28	1	31	71
% App. Total	0	0	0	0	0	90.3	9.7		66.7	0	33.3		6.5	90.3	3.2		
PHF	.000	.000	.000	.000	.000	.778	.375	.705	.750	.000	.750	.750	.500	.636	.250	.646	.710
Passenger +	0	0	0	0	0	28	3	31	6	0	3	9	2	26	1	29	69
% Passenger +	0	0	0	0	0	100	100	100	100	0	100	100	100	92.9	100	93.5	97.2
Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	2	0	2	2
% Heavy	0	0	0	0	0	0	0	0	0	0	0	0	0	7.1	0	6.5	2.8



# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623j  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Alameda St Southbound			Annapolis St Westbound			Alameda St Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
02:00 PM	34	2	36	1	0	1	1	25	26	63
02:15 PM	50	0	50	1	0	1	0	41	41	92
02:30 PM	42	3	45	0	0	0	0	42	42	87
02:45 PM	52	10	62	0	0	0	3	59	62	124
<b>Total</b>	<b>178</b>	<b>15</b>	<b>193</b>	<b>2</b>	<b>0</b>	<b>2</b>	<b>4</b>	<b>167</b>	<b>171</b>	<b>366</b>
03:00 PM	77	16	93	3	0	3	4	54	58	154
03:15 PM	65	7	72	0	0	0	3	45	48	120
03:30 PM	56	1	57	0	0	0	1	34	35	92
03:45 PM	58	0	58	2	1	3	0	39	39	100
<b>Total</b>	<b>256</b>	<b>24</b>	<b>280</b>	<b>5</b>	<b>1</b>	<b>6</b>	<b>8</b>	<b>172</b>	<b>180</b>	<b>466</b>
04:00 PM	58	2	60	1	1	2	2	47	49	111
04:15 PM	56	2	58	2	1	3	2	51	53	114
04:30 PM	55	2	57	1	0	1	3	58	61	119
04:45 PM	66	2	68	1	0	1	1	46	47	116
<b>Total</b>	<b>235</b>	<b>8</b>	<b>243</b>	<b>5</b>	<b>2</b>	<b>7</b>	<b>8</b>	<b>202</b>	<b>210</b>	<b>460</b>
05:00 PM	77	1	78	1	0	1	1	51	52	131
05:15 PM	64	2	66	0	0	0	2	43	45	111
05:30 PM	58	2	60	1	1	2	1	55	56	118
05:45 PM	54	4	58	0	0	0	8	65	73	131
<b>Total</b>	<b>253</b>	<b>9</b>	<b>262</b>	<b>2</b>	<b>1</b>	<b>3</b>	<b>12</b>	<b>214</b>	<b>226</b>	<b>491</b>
<b>Grand Total</b>	<b>922</b>	<b>56</b>	<b>978</b>	<b>14</b>	<b>4</b>	<b>18</b>	<b>32</b>	<b>755</b>	<b>787</b>	<b>1783</b>
<b>Apprch %</b>	94.3	5.7		77.8	22.2		4.1	95.9		
<b>Total %</b>	51.7	3.1	54.9	0.8	0.2	1	1.8	42.3	44.1	
<b>Passenger +</b>	902	55	957	13	4	17	31	748	779	1753
<b>% Passenger +</b>	97.8	98.2	97.9	92.9	100	94.4	96.9	99.1	99	98.3
<b>Heavy</b>	20	1	21	1	0	1	1	7	8	30
<b>% Heavy</b>	2.2	1.8	2.1	7.1	0	5.6	3.1	0.9	1	1.7

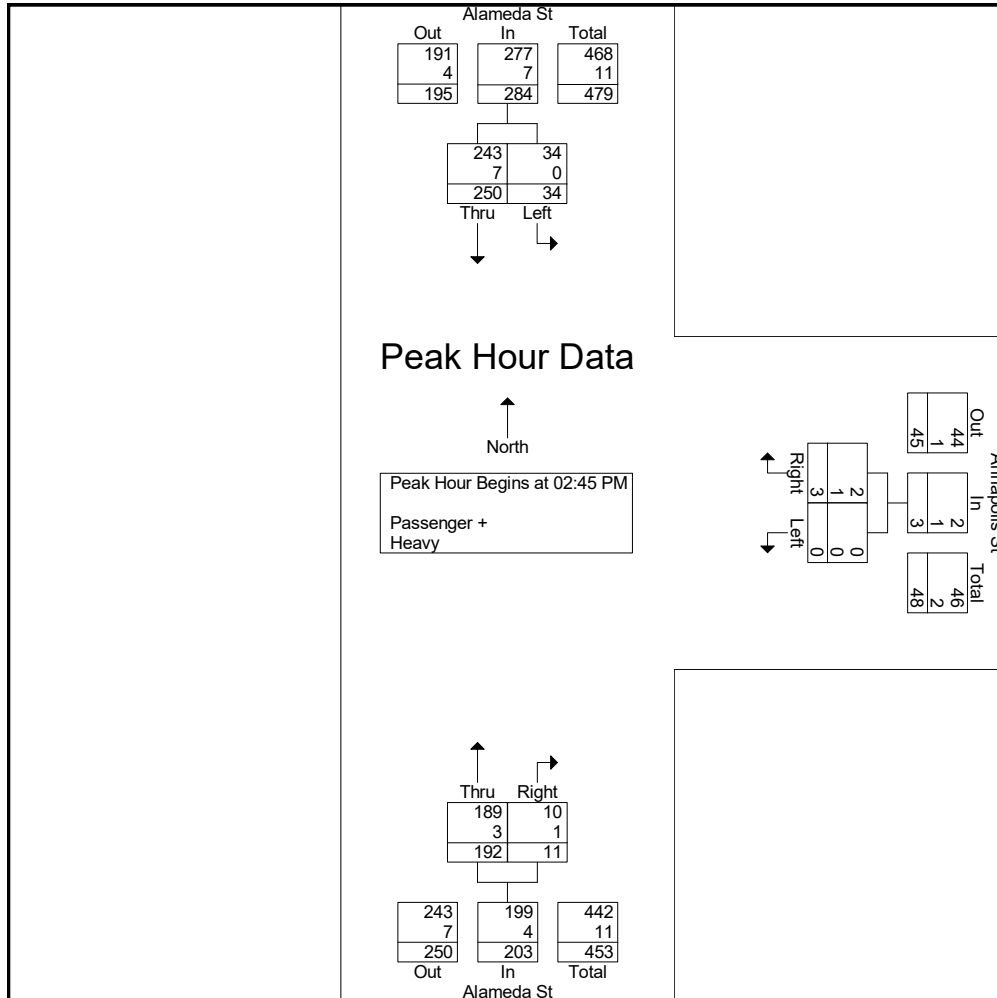
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623j  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Alameda St Southbound			Annapolis St Westbound			Alameda St Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 02:00 PM to 03:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 02:45 PM										
02:45 PM	52	10	62	0	0	0	3	<b>59</b>	<b>62</b>	124
03:00 PM	<b>77</b>	<b>16</b>	<b>93</b>	<b>3</b>	0	<b>3</b>	<b>4</b>	54	<b>58</b>	<b>154</b>
03:15 PM	65	7	72	0	0	0	3	45	48	120
03:30 PM	56	1	57	0	0	0	1	34	35	92
Total Volume	250	34	284	3	0	3	11	192	203	490
% App. Total	88	12		100	0		5.4	94.6		
PHF	.812	.531	.763	.250	.000	.250	.688	.814	.819	.795
Passenger +	243	34	277	2	0	2	10	189	199	478
% Passenger +	97.2	100	97.5	66.7	0	66.7	90.9	98.4	98.0	97.6
Heavy	7	0	7	1	0	1	1	3	4	12
% Heavy	2.8	0	2.5	33.3	0	33.3	9.1	1.6	2.0	2.4



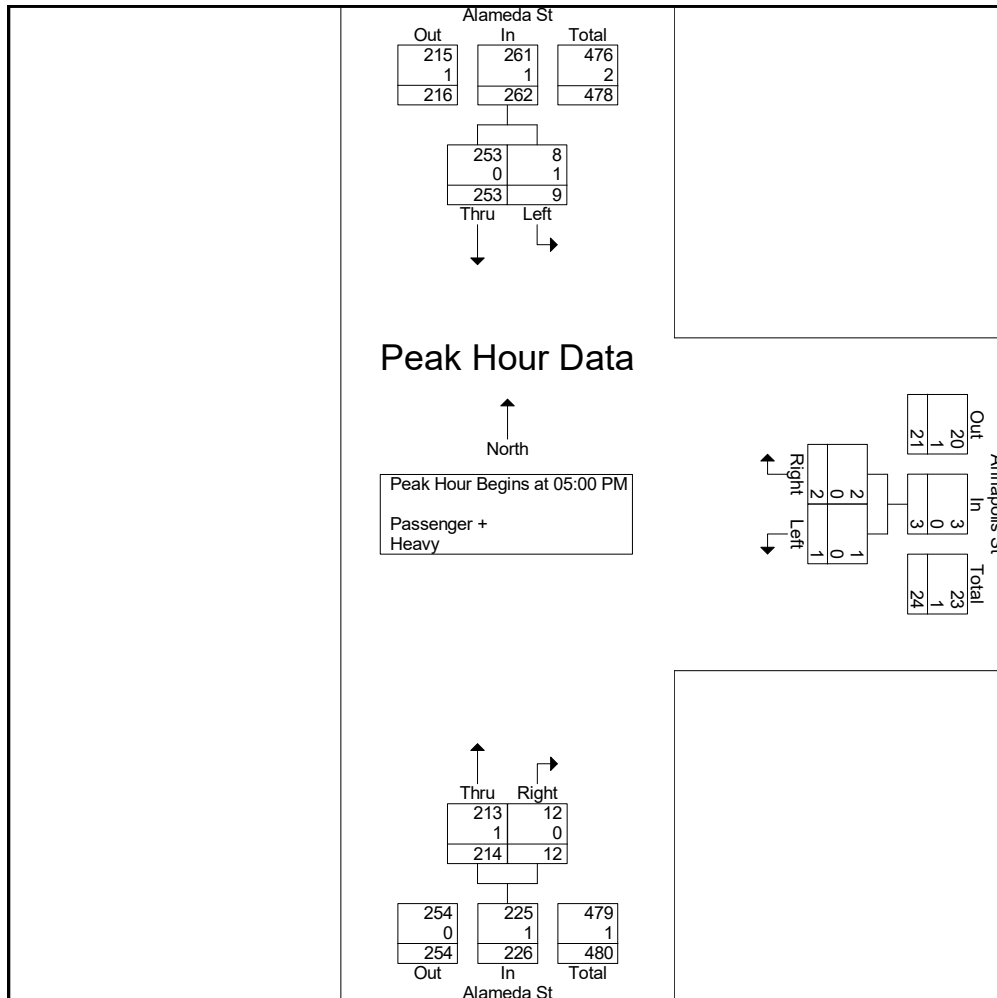
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623j  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 3

Start Time	Alameda St Southbound			Annapolis St Westbound			Alameda St Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 05:00 PM										
05:00 PM	77	1	78	1	0	1	1	51	52	131
05:15 PM	64	2	66	0	0	0	2	43	45	111
05:30 PM	58	2	60	1	1	2	1	55	56	118
05:45 PM	54	4	58	0	0	0	8	65	73	131
<b>Total Volume</b>	253	9	262	2	1	3	12	214	226	491
<b>% App. Total</b>	96.6	3.4		66.7	33.3		5.3	94.7		
PHF	.821	.563	.840	.500	.250	.375	.375	.823	.774	.937
Passenger +	253	8	261	2	1	3	12	213	225	489
% Passenger +	100	88.9	99.6	100	100	100	100	99.5	99.6	99.6
Heavy	0	1	1	0	0	0	0	1	1	2
% Heavy	0	11.1	0.4	0	0	0	0	0.5	0.4	0.4



# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623f  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 1

Groups Printed- Passenger + - Heavy

Start Time	Alameda Ave Southbound			School Access Westbound			Alameda Ave Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
02:00 PM	37	0	37	0	0	0	0	26	26	63
02:15 PM	51	1	52	0	0	0	3	39	42	94
02:30 PM	45	1	46	0	0	0	2	40	42	88
02:45 PM	60	5	65	0	1	1	5	52	57	123
<b>Total</b>	<b>193</b>	<b>7</b>	<b>200</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>10</b>	<b>157</b>	<b>167</b>	<b>368</b>
03:00 PM	83	4	87	1	8	9	8	49	57	153
03:15 PM	66	9	75	2	7	9	0	45	45	129
03:30 PM	53	4	57	2	4	6	1	31	32	95
03:45 PM	55	6	61	1	1	2	2	42	44	107
<b>Total</b>	<b>257</b>	<b>23</b>	<b>280</b>	<b>6</b>	<b>20</b>	<b>26</b>	<b>11</b>	<b>167</b>	<b>178</b>	<b>484</b>
04:00 PM	59	1	60	2	2	4	1	47	48	112
04:15 PM	59	1	60	0	0	0	0	52	52	112
04:30 PM	55	2	57	0	1	1	0	58	58	116
04:45 PM	69	1	70	0	0	0	2	45	47	117
<b>Total</b>	<b>242</b>	<b>5</b>	<b>247</b>	<b>2</b>	<b>3</b>	<b>5</b>	<b>3</b>	<b>202</b>	<b>205</b>	<b>457</b>
05:00 PM	77	1	78	1	0	1	2	51	53	132
05:15 PM	65	3	68	0	0	0	0	41	41	109
05:30 PM	58	4	62	2	0	2	1	54	55	119
05:45 PM	56	6	62	0	1	1	5	60	65	128
<b>Total</b>	<b>256</b>	<b>14</b>	<b>270</b>	<b>3</b>	<b>1</b>	<b>4</b>	<b>8</b>	<b>206</b>	<b>214</b>	<b>488</b>
<b>Grand Total</b>	<b>948</b>	<b>49</b>	<b>997</b>	<b>11</b>	<b>25</b>	<b>36</b>	<b>32</b>	<b>732</b>	<b>764</b>	<b>1797</b>
Apprch %	95.1	4.9		30.6	69.4		4.2	95.8		
Total %	52.8	2.7	55.5	0.6	1.4	2	1.8	40.7	42.5	
Passenger +	924	49	973	11	25	36	32	724	756	1765
% Passenger +	97.5	100	97.6	100	100	100	100	98.9	99	98.2
Heavy	24	0	24	0	0	0	0	8	8	32
% Heavy	2.5	0	2.4	0	0	0	0	1.1	1	1.8

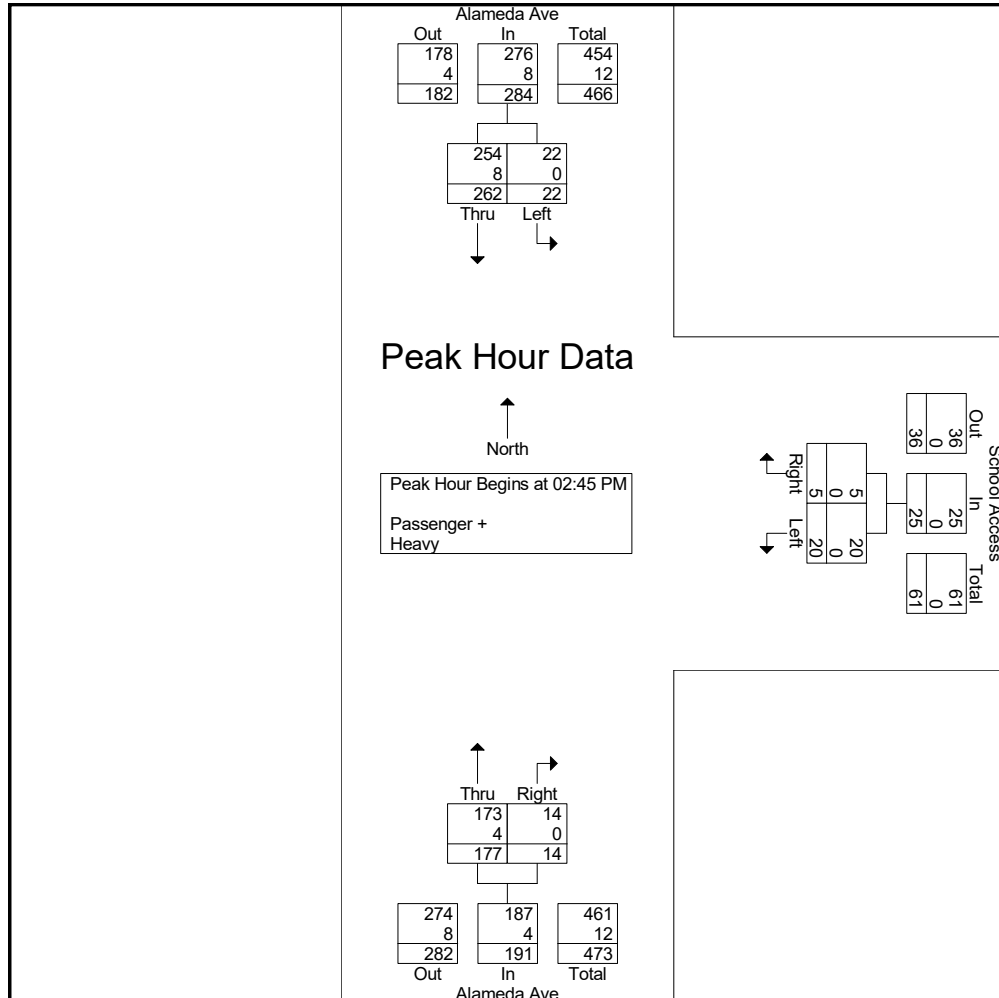
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623f  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 2

Start Time	Alameda Ave Southbound			School Access Westbound			Alameda Ave Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 02:00 PM to 03:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 02:45 PM										
02:45 PM	60	5	65	0	1	1	5	52	57	123
03:00 PM	83	4	87	1	8	9	8	49	57	153
03:15 PM	66	9	75	2	7	9	0	45	45	129
03:30 PM	53	4	57	2	4	6	1	31	32	95
Total Volume	262	22	284	5	20	25	14	177	191	500
% App. Total	92.3	7.7		20	80		7.3	92.7		
PHF	.789	.611	.816	.625	.625	.694	.438	.851	.838	.817
Passenger +	254	22	276	5	20	25	14	173	187	488
% Passenger +	96.9	100	97.2	100	100	100	100	97.7	97.9	97.6
Heavy	8	0	8	0	0	0	0	4	4	12
% Heavy	3.1	0	2.8	0	0	0	0	2.3	2.1	2.4



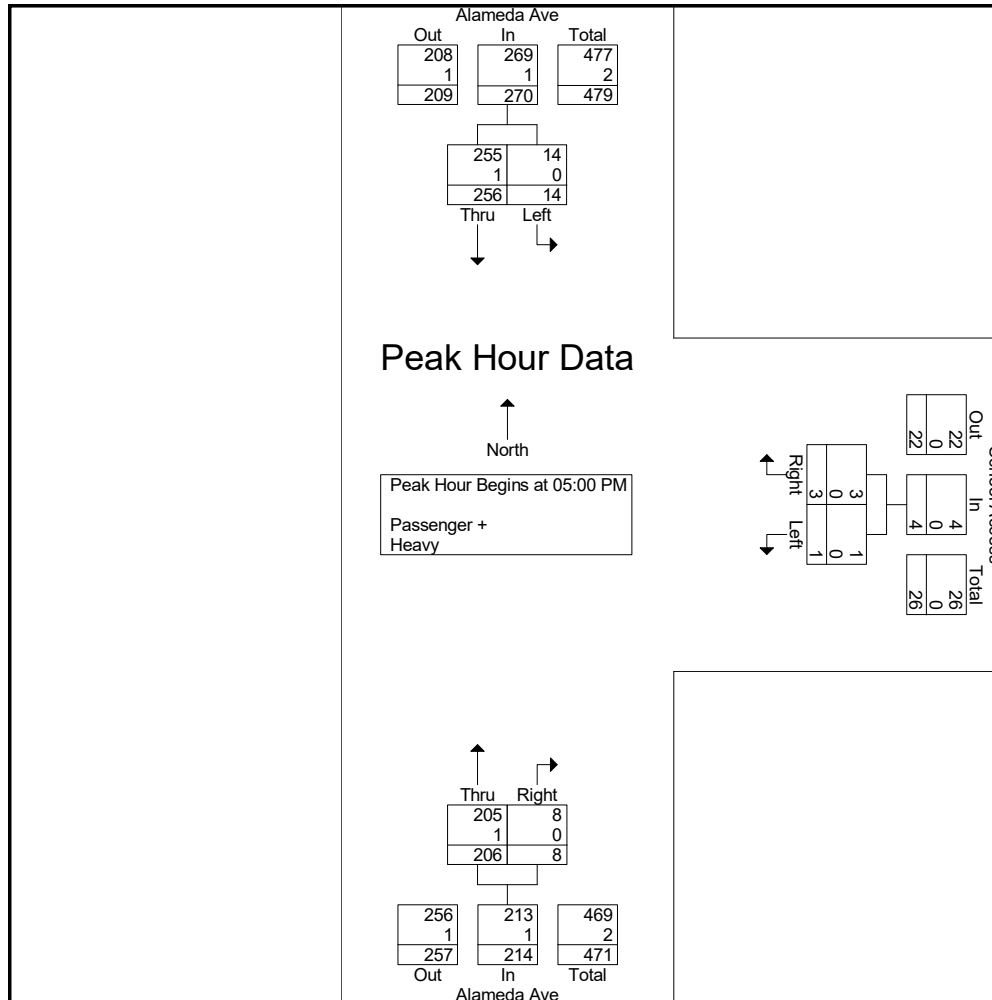
# Exhibit E

# Heath & Associates

PO Box 397 Puyallup, WA 98371

File Name : 5623f  
 Site Code : 00005623  
 Start Date : 6/12/2025  
 Page No : 3

Start Time	Alameda Ave Southbound			School Access Westbound			Alameda Ave Northbound			Int. Total
	Thru	Left	App. Total	Right	Left	App. Total	Right	Thru	App. Total	
Peak Hour Analysis From 04:00 PM to 05:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 05:00 PM										
05:00 PM	77	1	78	1	0	1	2	51	53	132
05:15 PM	65	3	68	0	0	0	0	41	41	109
05:30 PM	58	4	62	2	0	2	1	54	55	119
05:45 PM	56	6	62	0	1	1	5	60	65	128
<b>Total Volume</b>	256	14	270	3	1	4	8	206	214	488
<b>% App. Total</b>	94.8	5.2		75	25		3.7	96.3		
PHF	.831	.583	.865	.375	.250	.500	.400	.858	.823	.924
Passenger +	255	14	269	3	1	4	8	205	213	486
% Passenger +	99.6	100	99.6	100	100	100	100	99.5	99.5	99.6
Heavy	1	0	1	0	0	0	0	1	1	2
% Heavy	0.4	0	0.4	0	0	0	0	0.5	0.5	0.4



# Exhibit E

## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

*APPENDIX*  
ITE Trip Generation Worksheets



# Exhibit E

## Elementary School (520)

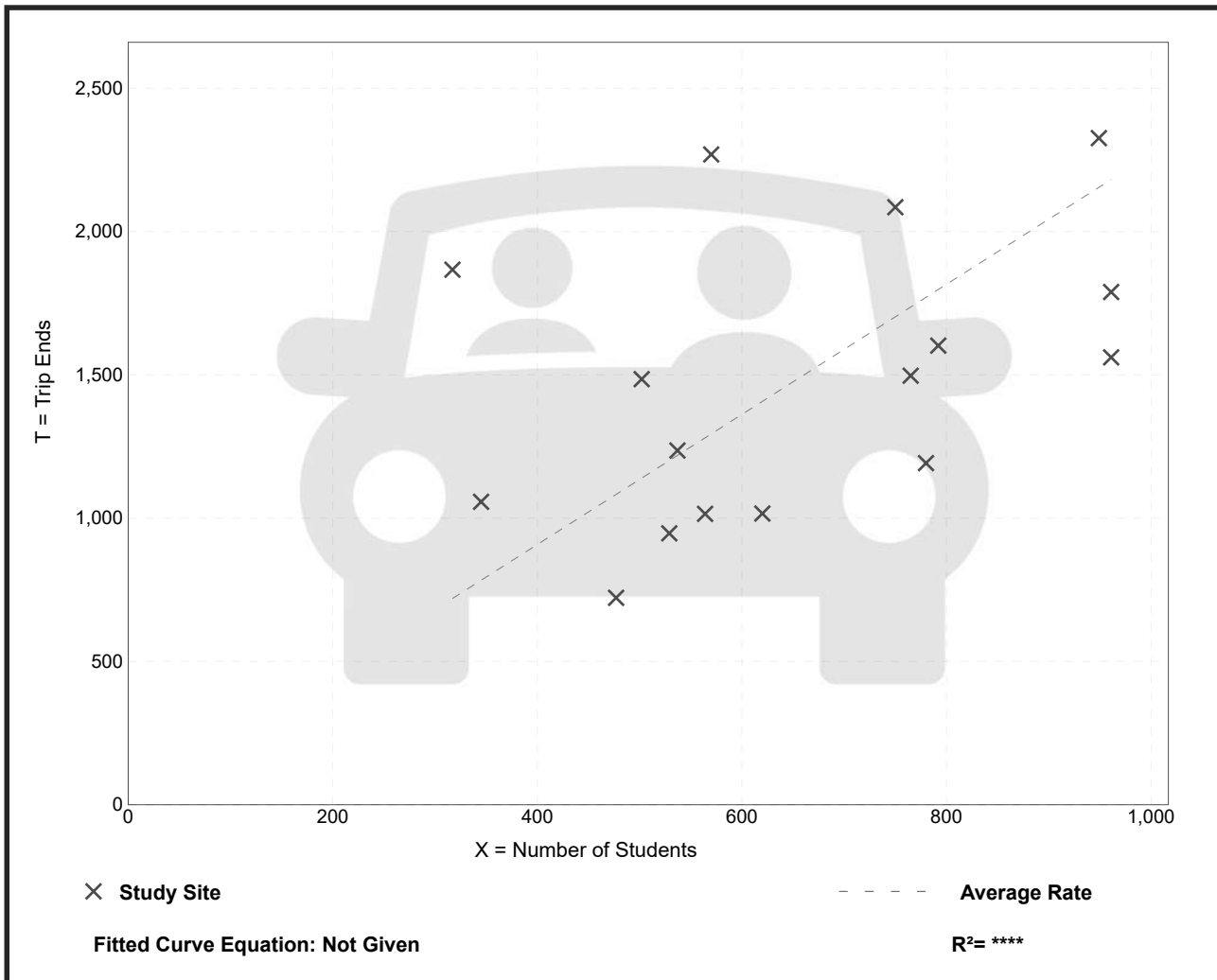
**Vehicle Trip Ends vs: Students**  
**On a: Weekday**

**Setting/Location: General Urban/Suburban**  
Number of Studies: 16  
Avg. Num. of Students: 651  
Directional Distribution: 50% entering, 50% exiting

### Vehicle Trip Generation per Student

Average Rate	Range of Rates	Standard Deviation
2.27	1.51 - 5.89	0.93

### Data Plot and Equation



# Exhibit E

## Elementary School (520)

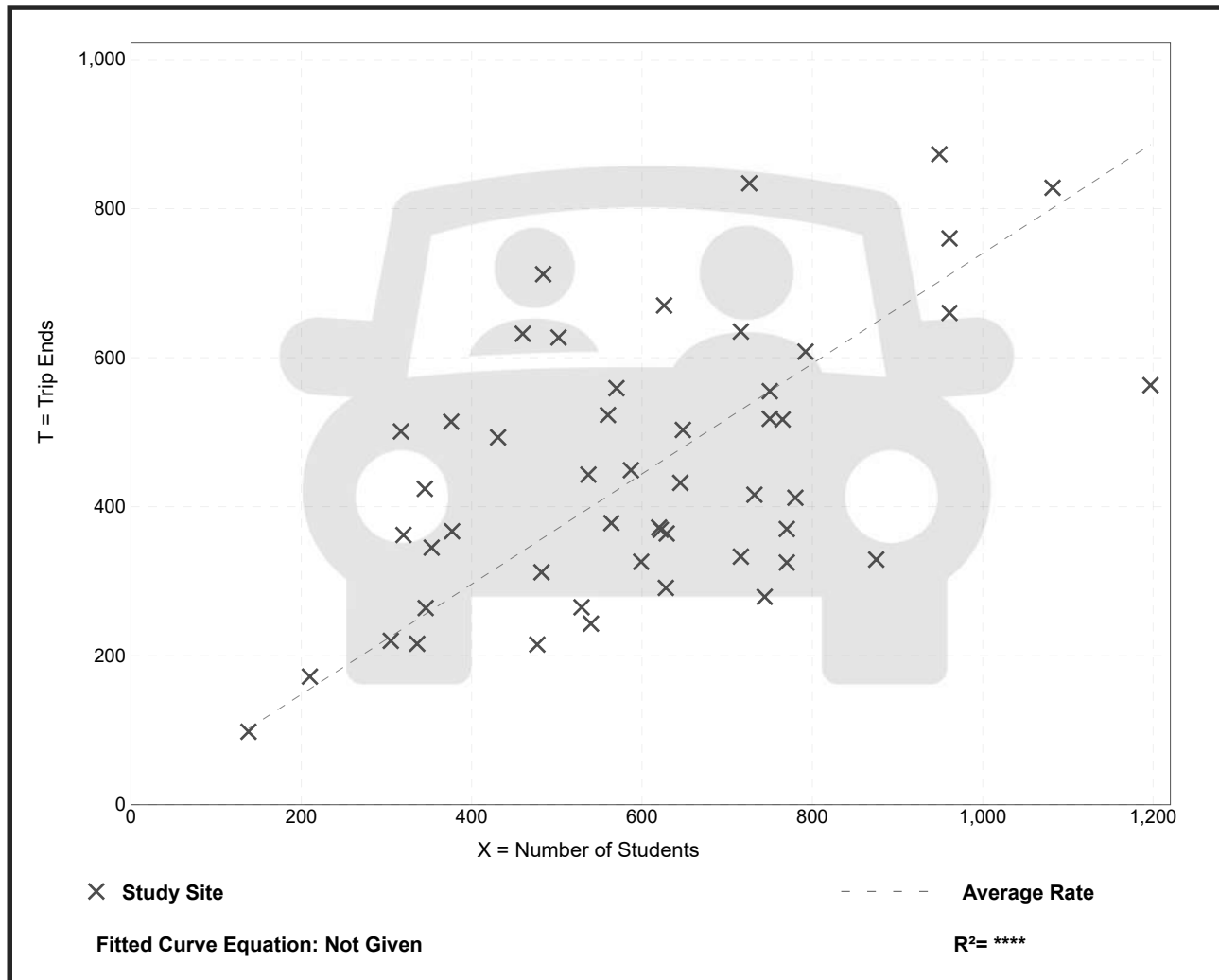
**Vehicle Trip Ends vs: Students**  
**On a: Weekday,**  
**AM Peak Hour of Generator**

**Setting/Location: General Urban/Suburban**  
 Number of Studies: 50  
 Avg. Num. of Students: 604  
 Directional Distribution: 54% entering, 46% exiting

### Vehicle Trip Generation per Student

Average Rate	Range of Rates	Standard Deviation
0.74	0.38 - 1.58	0.28

### Data Plot and Equation



# Exhibit E

## Elementary School (520)

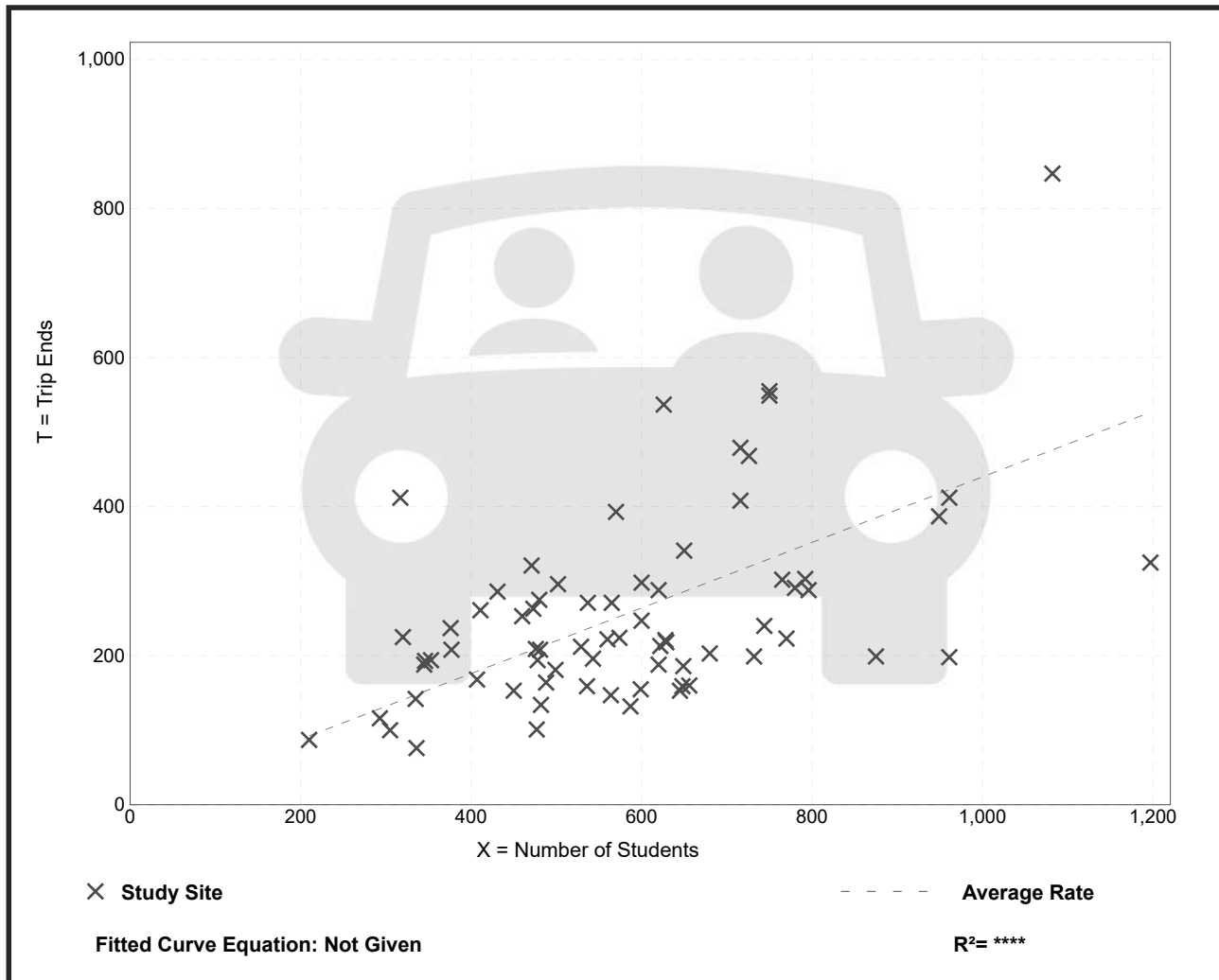
**Vehicle Trip Ends vs: Students**  
**On a: Weekday,**  
**PM Peak Hour of Generator**

**Setting/Location: General Urban/Suburban**  
 Number of Studies: 71  
 Avg. Num. of Students: 584  
 Directional Distribution: 46% entering, 54% exiting

### Vehicle Trip Generation per Student

Average Rate	Range of Rates	Standard Deviation
0.44	0.21 - 1.30	0.18

### Data Plot and Equation



# Exhibit E

## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

### *APPENDIX*

Level of Service Worksheets - Existing Conditions



# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Existing  
AM Peak

Intersection						
Int Delay, s/veh	0.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↖		↗			↘
Traffic Vol, veh/h	1	7	233	21	27	152
Future Vol, veh/h	1	7	233	21	27	152
Conflicting Peds, #/hr	2	3	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	7	248	22	29	162

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	480	262	0	0	270
Stage 1	259	-	-	-	-
Stage 2	221	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	545	777	-	-	1293
Stage 1	784	-	-	-	-
Stage 2	816	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	530	774	-	-	1293
Mov Cap-2 Maneuver	530	-	-	-	-
Stage 1	784	-	-	-	-
Stage 2	794	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	9.98	0	1.18
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	732	272
HCM Lane V/C Ratio	-	-	0.012	0.022
HCM Ctrl Dly (s/v)	-	-	10	7.8
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	0	0.1

Scenario 1. Existing AM Peak

Synchro 12 Report  
Page 1

# Exhibit E

## HCM 7th TWSC 2: Alameda Avenue & Parking Lot Access

Existing  
AM Peak

Intersection						
Int Delay, s/veh	0.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	12	3	209	31	20	167
Future Vol, veh/h	12	3	209	31	20	167
Conflicting Peds, #/hr	0	1	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	3	227	34	22	182

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	474	250	0	0	266
Stage 1	249	-	-	-	-
Stage 2	225	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	549	789	-	-	1298
Stage 1	792	-	-	-	-
Stage 2	812	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	536	784	-	-	1292
Mov Cap-2 Maneuver	536	-	-	-	-
Stage 1	789	-	-	-	-
Stage 2	797	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	11.47	0	0.84
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	572	193
HCM Lane V/C Ratio	-	-	0.028	0.017
HCM Ctrl Dly (s/v)	-	-	11.5	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.1	0.1

Scenario 1. Existing AM Peak

Synchro 12 Report  
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# Exhibit E

HCM 7th TWSC  
3: Alameda Avenue & Driveway/Elm Tree Lane

Existing  
AM Peak

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	22	0	23	0	196	16	14	165	0
Future Vol, veh/h	0	0	0	22	0	23	0	196	16	14	165	0
Conflicting Peds, #/hr	0	0	0	0	0	0	12	0	8	8	0	12
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	23	0	24	0	209	17	15	176	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	426	451	188	430	442	225	188	0	0	234	0	0
Stage 1	217	217	-	225	225	-	-	-	-	-	-	-
Stage 2	209	234	-	205	217	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	539	504	854	535	510	814	1387	-	-	1334	-	-
Stage 1	785	723	-	778	718	-	-	-	-	-	-	-
Stage 2	794	711	-	797	723	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	510	488	845	524	494	808	1371	-	-	1324	-	-
Mov Cap-2 Maneuver	510	488	-	524	494	-	-	-	-	-	-	-
Stage 1	766	706	-	772	712	-	-	-	-	-	-	-
Stage 2	769	706	-	787	706	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Ctrl Dly, s/v	0		11.09		0		0.61	
HCM LOS	A		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1371	-	-	-	-	639	141	-
HCM Lane V/C Ratio	-	-	-	-	-	0.075	0.011	-
HCM Ctrl Dly (s/v)	0	-	-	0	11.1	7.8	0	-
HCM Lane LOS	A	-	-	A	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	-	0.2	0	-	-

Scenario 1. Existing AM Peak

Synchro 12 Report  
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# Exhibit E

## HCM 7th TWSC 4: Bus Access & Elm Tree Lane

Existing  
AM Peak

Intersection							
Int Delay, s/veh	2.9						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↔			↔	↔	↔	
Traffic Vol, veh/h	30	0	1	27	18	10	
Future Vol, veh/h	30	0	1	27	18	10	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	0	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	84	84	84	84	84	84	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	36	0	1	32	21	12	
Major/Minor	Major1	Major2	Minor1				
Conflicting Flow All	0	0	36	0	70	36	
Stage 1	-	-	-	-	36	-	
Stage 2	-	-	-	-	35	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1575	-	934	1037	
Stage 1	-	-	-	-	987	-	
Stage 2	-	-	-	-	988	-	
Platoon blocked, %	-	-	-	-	-	-	
Mov Cap-1 Maneuver	-	-	1575	-	933	1037	
Mov Cap-2 Maneuver	-	-	-	-	933	-	
Stage 1	-	-	-	-	987	-	
Stage 2	-	-	-	-	987	-	
Approach	EB	WB	NB				
HCM Ctrl Dly, s/v	0	0.26	8.79				
HCM LOS				A			
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT	
Capacity (veh/h)	933	1037	-	-	64	-	
HCM Lane V/C Ratio	0.023	0.011	-	-	0.001	-	
HCM Ctrl Dly (s/v)	8.9	8.5	-	-	7.3	0	
HCM Lane LOS	A	A	-	-	A	A	
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	

Scenario 1. Existing AM Peak

Synchro 12 Report  
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# Exhibit E

HCM 7th TWSC  
5: Annapolis Street & Elm Tree Lane

Existing  
AM Peak

Intersection						
Int Delay, s/veh	4.2					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	40	0	1	26	2	53
Future Vol, veh/h	40	0	1	26	2	53
Conflicting Peds, #/hr	0	27	27	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	71	71	71	71	71	71
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	56	0	1	37	3	75

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	83	0	123
Stage 1	-	-	-	-	83
Stage 2	-	-	-	-	39
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	1514	-	872
Stage 1	-	-	-	-	940
Stage 2	-	-	-	-	983
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1475	-	849
Mov Cap-2 Maneuver	-	-	-	-	849
Stage 1	-	-	-	-	916
Stage 2	-	-	-	-	982

Approach	EB	WB	NB
HCM Ctrl Dly, s/v	0	0.28	9.14
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	947	-	-	67	-
HCM Lane V/C Ratio	0.082	-	-	0.001	-
HCM Ctrl Dly (s/v)	9.1	-	-	7.4	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0.3	-	-	0	-

Scenario 1. Existing AM Peak

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# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Existing  
PM Peak School

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	0	3	192	11	34	253
Future Vol, veh/h	0	3	192	11	34	253
Conflicting Peds, #/hr	1	2	0	3	3	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	4	240	14	43	316

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	652	252	0	0	257	0
Stage 1	250	-	-	-	-	-
Stage 2	402	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	433	787	-	-	1308	-
Stage 1	792	-	-	-	-	-
Stage 2	675	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	414	783	-	-	1304	-
Mov Cap-2 Maneuver	414	-	-	-	-	-
Stage 1	789	-	-	-	-	-
Stage 2	648	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	9.62	0	0.93
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	783	213
HCM Lane V/C Ratio	-	-	0.005	0.033
HCM Ctrl Dly (s/v)	-	-	9.6	7.9
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	0	0.1

Scenario 2. Existing PM Peak School

Synchro 12 Report  
Page 1

# Exhibit E

## HCM 7th TWSC 2: Alameda Avenue & Parking Lot Access

Existing  
PM Peak School

Intersection						
Int Delay, s/veh	1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	T		T			T
Traffic Vol, veh/h	20	5	181	14	22	267
Future Vol, veh/h	20	5	181	14	22	267
Conflicting Peds, #/hr	0	1	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	24	6	221	17	27	326

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	613	234	0	0	242
Stage 1	233	-	-	-	-
Stage 2	379	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	456	805	-	-	1325
Stage 1	805	-	-	-	-
Stage 2	692	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	443	801	-	-	1320
Mov Cap-2 Maneuver	443	-	-	-	-
Stage 1	802	-	-	-	-
Stage 2	675	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	12.89	0	0.59
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	487	137
HCM Lane V/C Ratio	-	-	0.063	0.02
HCM Ctrl Dly (s/v)	-	-	12.9	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.2	0.1

# Exhibit E

HCM 7th TWSC  
3: Alameda Avenue & Driveway/Elm Tree Lane

Existing  
PM Peak School

Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	1	0	0	28	0	14	0	168	18	10	261	0
Future Vol, veh/h	1	0	0	28	0	14	0	168	18	10	261	0
Conflicting Peds, #/hr	0	0	0	0	0	0	6	0	4	4	0	6
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	0	33	0	16	0	195	21	12	303	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	528	553	309	537	543	210	309	0	0	220	0	0
Stage 1	333	333	-	210	210	-	-	-	-	-	-	-
Stage 2	195	220	-	327	333	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	461	441	731	455	447	830	1251	-	-	1349	-	-
Stage 1	681	644	-	792	729	-	-	-	-	-	-	-
Stage 2	806	721	-	686	644	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	445	432	726	449	438	827	1244	-	-	1344	-	-
Mov Cap-2 Maneuver	445	432	-	449	438	-	-	-	-	-	-	-
Stage 1	670	634	-	789	726	-	-	-	-	-	-	-
Stage 2	791	718	-	679	634	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Ctrl Dly, s/v	13.12		12.49		0		0.28	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1244	-	-	445	529	66	-	-
HCM Lane V/C Ratio	-	-	-	0.003	0.092	0.009	-	-
HCM Ctrl Dly (s/v)	0	-	-	13.1	12.5	7.7	0	-
HCM Lane LOS	A	-	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0	0.3	0	-	-

Scenario 2. Existing PM Peak School

Synchro 12 Report  
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# Exhibit E

## HCM 7th TWSC 4: Bus Access & Elm Tree Lane

Existing  
PM Peak School

Intersection							
Int Delay, s/veh	2.4						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↔			↔	↔	↔	
Traffic Vol, veh/h	27	1	2	31	11	10	
Future Vol, veh/h	27	1	2	31	11	10	
Conflicting Peds, #/hr	0	5	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	0	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	86	86	86	86	86	86	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	31	1	2	36	13	12	
Major/Minor	Major1	Major2	Minor1				
Conflicting Flow All	0	0	38	0	78	37	
Stage 1	-	-	-	-	37	-	
Stage 2	-	-	-	-	41	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1573	-	925	1035	
Stage 1	-	-	-	-	985	-	
Stage 2	-	-	-	-	982	-	
Platoon blocked, %	-	-	-	-	-	-	
Mov Cap-1 Maneuver	-	-	1565	-	919	1030	
Mov Cap-2 Maneuver	-	-	-	-	919	-	
Stage 1	-	-	-	-	981	-	
Stage 2	-	-	-	-	980	-	
Approach	EB	WB	NB				
HCM Ctrl Dly, s/v	0	0.44	8.76				
HCM LOS				A			
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT	
Capacity (veh/h)	919	1030	-	-	109	-	
HCM Lane V/C Ratio	0.014	0.011	-	-	0.001	-	
HCM Ctrl Dly (s/v)	9	8.5	-	-	7.3	0	
HCM Lane LOS	A	A	-	-	A	A	
HCM 95th %tile Q(veh)	0	0	-	-	0	-	

# Exhibit E

HCM 7th TWSC  
5: Annapolis Street & Elm Tree Lane

Existing  
PM Peak School

Intersection						
Int Delay, s/veh	5.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	36	1	2	27	6	41
Future Vol, veh/h	36	1	2	27	6	41
Conflicting Peds, #/hr	0	189	189	0	0	68
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	55	55	55	55	55	55
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	65	2	4	49	11	75

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	256	0	312 323
Stage 1	-	-	-	-	255 -
Stage 2	-	-	-	-	56 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1309	-	681 718
Stage 1	-	-	-	-	787 -
Stage 2	-	-	-	-	966 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1073	-	556 550
Mov Cap-2 Maneuver	-	-	-	-	556 -
Stage 1	-	-	-	-	646 -
Stage 2	-	-	-	-	963 -

Approach	EB	WB	NB
HCM Ctrl Dly, s/v	0	0.58	12.73
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	551	-	-	124	-
HCM Lane V/C Ratio	0.155	-	-	0.003	-
HCM Ctrl Dly (s/v)	12.7	-	-	8.4	0
HCM Lane LOS	B	-	-	A	A
HCM 95th %tile Q(veh)	0.5	-	-	0	-

# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Existing  
PM Peak Street

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	1	2	214	12	9	245
Future Vol, veh/h	1	2	214	12	9	245
Conflicting Peds, #/hr	2	3	0	5	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	2	228	13	10	261

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	521	242	0	0	245
Stage 1	239	-	-	-	-
Stage 2	282	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	516	797	-	-	1321
Stage 1	801	-	-	-	-
Stage 2	766	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	508	791	-	-	1314
Mov Cap-2 Maneuver	508	-	-	-	-
Stage 1	797	-	-	-	-
Stage 2	758	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	10.42	0	0.27
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	667	64
HCM Lane V/C Ratio	-	-	0.005	0.007
HCM Ctrl Dly (s/v)	-	-	10.4	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0

Scenario 3. Existing PM Peak Street

Synchro 12 Report  
Page 1

# Exhibit E

HCM 7th TWSC  
2: Alameda Avenue & Parking Lot Access

Existing  
PM Peak Street

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	1	3	208	8	14	253
Future Vol, veh/h	1	3	208	8	14	253
Conflicting Peds, #/hr	0	1	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	4	254	10	17	309

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	606	265	0	0	268	0
Stage 1	264	-	-	-	-	-
Stage 2	343	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	460	774	-	-	1295	-
Stage 1	781	-	-	-	-	-
Stage 2	719	-	-	-	-	-
Platoon blocked, %			-	-	-	-
Mov Cap-1 Maneuver	451	770	-	-	1289	-
Mov Cap-2 Maneuver	451	-	-	-	-	-
Stage 1	777	-	-	-	-	-
Stage 2	707	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	10.55	0	0.41
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	654	94
HCM Lane V/C Ratio	-	-	0.007	0.013
HCM Ctrl Dly (s/v)	-	-	10.5	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0

Scenario 3. Existing PM Peak Street

Synchro 12 Report  
Page 2

# Exhibit E

HCM 7th TWSC  
3: Alameda Avenue & Driveway/Elm Tree Lane

Existing  
PM Peak Street

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	1	0	0	24	0	3	0	193	18	13	243	0
Future Vol, veh/h	1	0	0	24	0	3	0	193	18	13	243	0
Conflicting Peds, #/hr	0	0	0	0	0	0	12	0	8	8	0	12
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	0	26	0	3	0	210	20	14	264	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	514	542	276	520	532	228	276	0	0	237	0	0
Stage 1	304	304	-	228	228	-	-	-	-	-	-	-
Stage 2	210	237	-	292	304	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	471	448	763	467	453	812	1287	-	-	1330	-	-
Stage 1	705	663	-	775	716	-	-	-	-	-	-	-
Stage 2	792	709	-	716	663	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	458	434	754	457	439	806	1272	-	-	1320	-	-
Mov Cap-2 Maneuver	458	434	-	457	439	-	-	-	-	-	-	-
Stage 1	688	647	-	769	710	-	-	-	-	-	-	-
Stage 2	789	703	-	707	647	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Ctrl Dly, s/v	12.88		12.98		0		0.39	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1272	-	-	458	480	91	-	-
HCM Lane V/C Ratio	-	-	-	0.002	0.061	0.011	-	-
HCM Ctrl Dly (s/v)	0	-	-	12.9	13	7.8	0	-
HCM Lane LOS	A	-	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0	0.2	0	-	-

Scenario 3. Existing PM Peak Street

Synchro 12 Report  
Page 3

# Exhibit E

HCM 7th TWSC  
4: Bus Access & Elm Tree Lane

Existing  
PM Peak Street

Intersection							
Int Delay, s/veh	1.4						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↔			↔	↔	↔	
Traffic Vol, veh/h	30	1	5	25	2	5	
Future Vol, veh/h	30	1	5	25	2	5	
Conflicting Peds, #/hr	0	7	7	0	7	1	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	0	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	84	84	84	84	84	84	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	36	1	6	30	2	6	
Major/Minor	Major1	Major2	Minor1				
Conflicting Flow All	0	0	44	0	92	44	
Stage 1	-	-	-	-	43	-	
Stage 2	-	-	-	-	49	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1565	-	908	1026	
Stage 1	-	-	-	-	979	-	
Stage 2	-	-	-	-	974	-	
Platoon blocked, %	-	-	-	-	-	-	
Mov Cap-1 Maneuver	-	-	1554	-	893	1018	
Mov Cap-2 Maneuver	-	-	-	-	893	-	
Stage 1	-	-	-	-	973	-	
Stage 2	-	-	-	-	964	-	
Approach	EB	WB	NB				
HCM Ctrl Dly, s/v	0	1.22	8.7				
HCM LOS				A			
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT	
Capacity (veh/h)	893	1018	-	-	300	-	
HCM Lane V/C Ratio	0.003	0.006	-	-	0.004	-	
HCM Ctrl Dly (s/v)	9	8.6	-	-	7.3	0	
HCM Lane LOS	A	A	-	-	A	A	
HCM 95th %tile Q(veh)	0	0	-	-	0	-	

Scenario 3. Existing PM Peak Street

Synchro 12 Report  
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# Exhibit E

HCM 7th TWSC  
5: Annapolis Street & Elm Tree Lane

Existing  
PM Peak Street

Intersection						
Int Delay, s/veh	1.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑	↑	
Traffic Vol, veh/h	33	2	3	25	5	4
Future Vol, veh/h	33	2	3	25	5	4
Conflicting Peds, #/hr	0	27	27	0	2	7
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	71	71	71	71	71	71
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	46	3	4	35	7	6

Major/Minor	Major1	Major2	Minor1	Minor2	Minor3
Conflicting Flow All	0	0	76	0	121
Stage 1	-	-	-	-	75
Stage 2	-	-	-	-	46
Critical Hdwy	-	-	4.12	-	6.42
Critical Hdwy Stg 1	-	-	-	-	5.42
Critical Hdwy Stg 2	-	-	-	-	5.42
Follow-up Hdwy	-	-	2.218	-	3.518
Pot Cap-1 Maneuver	-	-	1523	-	875
Stage 1	-	-	-	-	948
Stage 2	-	-	-	-	977
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1483	-	848
Mov Cap-2 Maneuver	-	-	-	-	848
Stage 1	-	-	-	-	924
Stage 2	-	-	-	-	972

Approach	EB	WB	NB
HCM Ctrl Dly, s/v	0	0.8	9.11
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	889	-	-	193	-
HCM Lane V/C Ratio	0.014	-	-	0.003	-
HCM Ctrl Dly (s/v)	9.1	-	-	7.4	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0	-	-	0	-

Scenario 3. Existing PM Peak Street

Synchro 12 Report  
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# Exhibit E

## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

### *APPENDIX*

Level of Service Worksheets - Forecast 2028 Without Project Conditions



# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Forecast Without Project  
AM Peak School

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	1	7	240	21	27	158
Future Vol, veh/h	1	7	240	21	27	158
Conflicting Peds, #/hr	2	3	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	7	255	22	29	168

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	494	269	0	0	278
Stage 1	266	-	-	-	-
Stage 2	228	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	535	769	-	-	1285
Stage 1	778	-	-	-	-
Stage 2	810	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	520	767	-	-	1285
Mov Cap-2 Maneuver	520	-	-	-	-
Stage 1	778	-	-	-	-
Stage 2	789	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	10.03	0	1.15
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	724	263
HCM Lane V/C Ratio	-	-	0.012	0.022
HCM Ctrl Dly (s/v)	-	-	10	7.9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0.1

Scenario 4. Forecast Without AM Peak

Synchro 12 Report  
Page 1

# Exhibit E

## HCM 7th TWSC 2: Alameda Avenue & Parking Lot Access

Forecast Without Project  
AM Peak School

Intersection						
Int Delay, s/veh	0.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	12	3	216	31	20	173
Future Vol, veh/h	12	3	216	31	20	173
Conflicting Peds, #/hr	0	1	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	13	3	235	34	22	188

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	488	258	0	0	273
Stage 1	257	-	-	-	-
Stage 2	232	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	539	781	-	-	1290
Stage 1	786	-	-	-	-
Stage 2	807	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	526	776	-	-	1284
Mov Cap-2 Maneuver	526	-	-	-	-
Stage 1	782	-	-	-	-
Stage 2	792	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	11.59	0	0.81
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	562	187
HCM Lane V/C Ratio	-	-	0.029	0.017
HCM Ctrl Dly (s/v)	-	-	11.6	7.9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.1	0.1

# Exhibit E

## HCM 7th TWSC 3: Alameda Avenue & Driveway/Elm Tree Lane

Forecast Without Project  
AM Peak School

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	24	0	24	0	203	16	14	170	0
Future Vol, veh/h	0	0	0	24	0	24	0	203	16	14	170	0
Conflicting Peds, #/hr	0	0	0	0	0	0	12	0	8	8	0	12
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	94	94	94	94	94	94	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	26	0	26	0	216	17	15	181	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	439	464	193	443	455	232	193	0	0	241	0	0
Stage 1	223	223	-	232	232	-	-	-	-	-	-	-
Stage 2	216	241	-	211	223	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	528	496	849	525	501	807	1380	-	-	1326	-	-
Stage 1	780	719	-	770	712	-	-	-	-	-	-	-
Stage 2	786	706	-	791	719	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	499	480	839	514	485	801	1365	-	-	1315	-	-
Mov Cap-2 Maneuver	499	480	-	514	485	-	-	-	-	-	-	-
Stage 1	761	702	-	765	707	-	-	-	-	-	-	-
Stage 2	761	701	-	781	702	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0	11.26	0	0.59
HCM LOS	A	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1365	-	-	-	626	137	-
HCM Lane V/C Ratio	-	-	-	-	0.082	0.011	-
HCM Ctrl Dly (s/v)	0	-	-	0	11.3	7.8	0
HCM Lane LOS	A	-	-	A	B	A	A
HCM 95th %tile Q(veh)	0	-	-	-	0.3	0	-

# Exhibit E

## HCM 7th TWSC 4: Bus Access & Elm Tree Lane

Forecast Without Project  
AM Peak School

Intersection						
Int Delay, s/veh	2.9					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↶			↷	↶	↷
Traffic Vol, veh/h	30	0	1	28	18	10
Future Vol, veh/h	30	0	1	28	18	10
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	36	0	1	33	21	12
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	36	0	71	36
Stage 1	-	-	-	-	36	-
Stage 2	-	-	-	-	36	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1575	-	933	1037
Stage 1	-	-	-	-	987	-
Stage 2	-	-	-	-	987	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1575	-	932	1037
Mov Cap-2 Maneuver	-	-	-	-	932	-
Stage 1	-	-	-	-	987	-
Stage 2	-	-	-	-	986	-
Approach	EB	WB	NB			
HCM Ctrl Dly, s/v	0	0.25	8.8			
HCM LOS				A		
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	932	1037	-	-	62	-
HCM Lane V/C Ratio	0.023	0.011	-	-	0.001	-
HCM Ctrl Dly (s/v)	9	8.5	-	-	7.3	0
HCM Lane LOS	A	A	-	-	A	A
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-

# Exhibit E

HCM 7th TWSC  
5: Annapolis Street & Elm Tree Lane

Forecast Without Project  
AM Peak School

Intersection						
Int Delay, s/veh	4.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↶			↷	↶	↷
Traffic Vol, veh/h	40	0	1	27	2	53
Future Vol, veh/h	40	0	1	27	2	53
Conflicting Peds, #/hr	0	27	27	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	71	71	71	71	71	71
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	56	0	1	38	3	75

Major/Minor	Major1	Major2	Minor1		
Conflicting Flow All	0	0	83	0	124 83
Stage 1	-	-	-	-	83 -
Stage 2	-	-	-	-	41 -
Critical Hdwy	-	-	4.12	-	6.42 6.22
Critical Hdwy Stg 1	-	-	-	-	5.42 -
Critical Hdwy Stg 2	-	-	-	-	5.42 -
Follow-up Hdwy	-	-	2.218	-	3.518 3.318
Pot Cap-1 Maneuver	-	-	1514	-	871 976
Stage 1	-	-	-	-	940 -
Stage 2	-	-	-	-	982 -
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1475	-	848 951
Mov Cap-2 Maneuver	-	-	-	-	848 -
Stage 1	-	-	-	-	916 -
Stage 2	-	-	-	-	981 -

Approach	EB	WB	NB
HCM Ctrl Dly, s/v	0	0.27	9.14
HCM LOS			A

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	947	-	-	64	-
HCM Lane V/C Ratio	0.082	-	-	0.001	-
HCM Ctrl Dly (s/v)	9.1	-	-	7.4	0
HCM Lane LOS	A	-	-	A	A
HCM 95th %tile Q(veh)	0.3	-	-	0	-

Scenario 4. Forecast Without AM Peak

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# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Forecast Without Project  
PM Peak School

Intersection						
Int Delay, s/veh	0.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	0	3	198	11	34	262
Future Vol, veh/h	0	3	198	11	34	262
Conflicting Peds, #/hr	1	2	0	3	3	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	4	248	14	43	328

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	671	259	0	0	264
Stage 1	257	-	-	-	-
Stage 2	414	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	422	779	-	-	1300
Stage 1	786	-	-	-	-
Stage 2	667	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	403	775	-	-	1296
Mov Cap-2 Maneuver	403	-	-	-	-
Stage 1	783	-	-	-	-
Stage 2	640	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	9.67	0	0.9
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	775	207
HCM Lane V/C Ratio	-	-	0.005	0.033
HCM Ctrl Dly (s/v)	-	-	9.7	7.9
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	0	0.1

Scenario 5. Forecast Without PM Peak School

Synchro 12 Report  
Page 1

# Exhibit E

## HCM 7th TWSC 2: Alameda Avenue & Parking Lot Access

Forecast Without Project  
PM Peak School

Intersection						
Int Delay, s/veh	1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	T		T			T
Traffic Vol, veh/h	20	5	187	14	22	276
Future Vol, veh/h	20	5	187	14	22	276
Conflicting Peds, #/hr	0	1	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	24	6	228	17	27	337

Major/Minor	Minor1	Major1	Major2		
Conflicting Flow All	631	242	0	0	249
Stage 1	241	-	-	-	-
Stage 2	390	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12
Critical Hdwy Stg 1	5.42	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218
Pot Cap-1 Maneuver	445	797	-	-	1317
Stage 1	799	-	-	-	-
Stage 2	684	-	-	-	-
Platoon blocked, %			-	-	-
Mov Cap-1 Maneuver	432	793	-	-	1311
Mov Cap-2 Maneuver	432	-	-	-	-
Stage 1	796	-	-	-	-
Stage 2	667	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	13.09	0	0.58
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	476	133
HCM Lane V/C Ratio	-	-	0.064	0.02
HCM Ctrl Dly (s/v)	-	-	13.1	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.2	0.1

# Exhibit E

## HCM 7th TWSC 3: Alameda Avenue & Driveway/Elm Tree Lane

Forecast Without Project  
PM Peak School

Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	1	0	0	29	0	14	0	173	19	10	269	0
Future Vol, veh/h	1	0	0	29	0	14	0	173	19	10	269	0
Conflicting Peds, #/hr	0	0	0	0	0	0	6	0	4	4	0	6
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	0	34	0	16	0	201	22	12	313	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	543	569	319	552	558	216	319	0	0	227	0	0
Stage 1	342	342	-	216	216	-	-	-	-	-	-	-
Stage 2	201	227	-	336	342	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	450	432	722	444	438	824	1241	-	-	1341	-	-
Stage 1	673	638	-	786	724	-	-	-	-	-	-	-
Stage 2	801	716	-	678	638	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	434	423	718	438	429	821	1234	-	-	1336	-	-
Mov Cap-2 Maneuver	434	423	-	438	429	-	-	-	-	-	-	-
Stage 1	662	628	-	783	721	-	-	-	-	-	-	-
Stage 2	785	713	-	671	628	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Ctrl Dly, s/v	13.31		12.72		0		0.28	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1234	-	-	434	516	65	-	-
HCM Lane V/C Ratio	-	-	-	0.003	0.097	0.009	-	-
HCM Ctrl Dly (s/v)	0	-	-	13.3	12.7	7.7	0	-
HCM Lane LOS	A	-	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0	0.3	0	-	-

# Exhibit E

## HCM 7th TWSC 4: Bus Access & Elm Tree Lane

Forecast Without Project  
PM Peak School

Intersection							
Int Delay, s/veh	2.4						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↔			↔	↔	↔	
Traffic Vol, veh/h	28	1	2	32	11	10	
Future Vol, veh/h	28	1	2	32	11	10	
Conflicting Peds, #/hr	0	5	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	0	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	86	86	86	86	86	86	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	33	1	2	37	13	12	
Major/Minor	Major1	Major2	Minor1				
Conflicting Flow All	0	0	39	0	80	38	
Stage 1	-	-	-	-	38	-	
Stage 2	-	-	-	-	42	-	
Critical Hdwy	-	-	4.12	-	6.42	6.22	
Critical Hdwy Stg 1	-	-	-	-	5.42	-	
Critical Hdwy Stg 2	-	-	-	-	5.42	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1571	-	922	1034	
Stage 1	-	-	-	-	984	-	
Stage 2	-	-	-	-	981	-	
Platoon blocked, %	-	-	-	-	-	-	
Mov Cap-1 Maneuver	-	-	1564	-	917	1029	
Mov Cap-2 Maneuver	-	-	-	-	917	-	
Stage 1	-	-	-	-	980	-	
Stage 2	-	-	-	-	979	-	
Approach	EB	WB	NB				
HCM Ctrl Dly, s/v	0	0.43	8.77				
HCM LOS				A			
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT	
Capacity (veh/h)	917	1029	-	-	106	-	
HCM Lane V/C Ratio	0.014	0.011	-	-	0.001	-	
HCM Ctrl Dly (s/v)	9	8.5	-	-	7.3	0	
HCM Lane LOS	A	A	-	-	A	A	
HCM 95th %tile Q(veh)	0	0	-	-	0	-	

# Exhibit E

## HCM 7th TWSC 5: Annapolis Street & Elm Tree Lane

Forecast Without Project  
PM Peak School

Intersection						
Int Delay, s/veh	5.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	37	1	2	28	6	41
Future Vol, veh/h	37	1	2	28	6	41
Conflicting Peds, #/hr	0	189	189	0	0	68
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	55	55	55	55	55	55
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	67	2	4	51	11	75
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	258	0	315	325
Stage 1	-	-	-	-	257	-
Stage 2	-	-	-	-	58	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1307	-	678	716
Stage 1	-	-	-	-	786	-
Stage 2	-	-	-	-	964	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1071	-	554	549
Mov Cap-2 Maneuver	-	-	-	-	554	-
Stage 1	-	-	-	-	644	-
Stage 2	-	-	-	-	961	-
Approach	EB	WB	NB			
HCM Ctrl Dly, s/v	0	0.56	12.75			
HCM LOS			B			
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)	550	-	-	120	-	
HCM Lane V/C Ratio	0.155	-	-	0.003	-	
HCM Ctrl Dly (s/v)	12.8	-	-	8.4	0	
HCM Lane LOS	B	-	-	A	A	
HCM 95th %tile Q(veh)	0.5	-	-	0	-	

# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Forecast Without Project  
PM Peak Street

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	1	2	220	12	9	253
Future Vol, veh/h	1	2	220	12	9	253
Conflicting Peds, #/hr	2	3	0	5	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	2	234	13	10	269

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	536	248	0	0	252	0
Stage 1	245	-	-	-	-	-
Stage 2	290	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	506	790	-	-	1314	-
Stage 1	795	-	-	-	-	-
Stage 2	759	-	-	-	-	-
Platoon blocked, %			-	-	-	-
Mov Cap-1 Maneuver	498	784	-	-	1307	-
Mov Cap-2 Maneuver	498	-	-	-	-	-
Stage 1	792	-	-	-	-	-
Stage 2	751	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	10.5	0	0.27
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	658	62
HCM Lane V/C Ratio	-	-	0.005	0.007
HCM Ctrl Dly (s/v)	-	-	10.5	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0

Scenario 6. Forecast Without PM Peak Street

Synchro 12 Report  
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# Exhibit E

## HCM 7th TWSC 2: Alameda Avenue & Parking Lot Access

Forecast Without Project  
PM Peak Street

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	1	3	214	8	14	261
Future Vol, veh/h	1	3	214	8	14	261
Conflicting Peds, #/hr	0	1	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	1	4	261	10	17	318

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	623	272	0	0	276	0
Stage 1	271	-	-	-	-	-
Stage 2	352	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	450	767	-	-	1287	-
Stage 1	775	-	-	-	-	-
Stage 2	712	-	-	-	-	-
Platoon blocked, %			-	-	-	-
Mov Cap-1 Maneuver	440	762	-	-	1281	-
Mov Cap-2 Maneuver	440	-	-	-	-	-
Stage 1	771	-	-	-	-	-
Stage 2	700	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	10.63	0	0.4
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	644	92
HCM Lane V/C Ratio	-	-	0.008	0.013
HCM Ctrl Dly (s/v)	-	-	10.6	7.8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0

# Exhibit E

## HCM 7th TWSC 3: Alameda Avenue & Driveway/Elm Tree Lane

Forecast Without Project  
PM Peak Street

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	1	0	0	25	0	3	0	198	19	13	250	1
Future Vol, veh/h	1	0	0	25	0	3	0	198	19	13	250	1
Conflicting Peds, #/hr	0	0	0	0	0	0	12	0	8	8	0	12
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	0	27	0	3	0	215	21	14	272	1

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	528	556	284	534	547	234	285	0	0	244	0	0
Stage 1	313	313	-	234	234	-	-	-	-	-	-	-
Stage 2	215	244	-	300	313	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	461	439	755	457	445	806	1277	-	-	1322	-	-
Stage 1	698	657	-	769	711	-	-	-	-	-	-	-
Stage 2	787	704	-	709	657	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	448	425	746	448	431	799	1263	-	-	1312	-	-
Mov Cap-2 Maneuver	448	425	-	448	431	-	-	-	-	-	-	-
Stage 1	681	641	-	764	706	-	-	-	-	-	-	-
Stage 2	784	699	-	700	641	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Ctrl Dly, s/v	13.05		13.19		0		0.38	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1263	-	-	448	470	89	-	-
HCM Lane V/C Ratio	-	-	-	0.002	0.065	0.011	-	-
HCM Ctrl Dly (s/v)	0	-	-	13	13.2	7.8	0	-
HCM Lane LOS	A	-	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0	0.2	0	-	-

# Exhibit E

## HCM 7th TWSC 4: Bus Access & Elm Tree Lane

Forecast Without Project  
PM Peak Street

Intersection						
Int Delay, s/veh	1.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↶			↷	↶	↷
Traffic Vol, veh/h	31	1	5	26	2	5
Future Vol, veh/h	31	1	5	26	2	5
Conflicting Peds, #/hr	0	7	7	0	7	1
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	37	1	6	31	2	6
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	45	0	94	46
Stage 1	-	-	-	-	45	-
Stage 2	-	-	-	-	50	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1563	-	905	1024
Stage 1	-	-	-	-	978	-
Stage 2	-	-	-	-	973	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1553	-	890	1016
Mov Cap-2 Maneuver	-	-	-	-	890	-
Stage 1	-	-	-	-	971	-
Stage 2	-	-	-	-	962	-
Approach	EB	WB	NB			
HCM Ctrl Dly, s/v	0	1.18	8.7			
HCM LOS			A			
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBT	EBR	WBL	WBT
Capacity (veh/h)	890	1016	-	-	290	-
HCM Lane V/C Ratio	0.003	0.006	-	-	0.004	-
HCM Ctrl Dly (s/v)	9.1	8.6	-	-	7.3	0
HCM Lane LOS	A	A	-	-	A	A
HCM 95th %tile Q(veh)	0	0	-	-	0	-

# Exhibit E

HCM 7th TWSC  
5: Annapolis Street & Elm Tree Lane

Forecast Without Project  
PM Peak Street

Intersection						
Int Delay, s/veh	1.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↔	↔	
Traffic Vol, veh/h	34	2	3	26	5	4
Future Vol, veh/h	34	2	3	26	5	4
Conflicting Peds, #/hr	0	27	27	0	2	7
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	71	71	71	71	71	71
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	48	3	4	37	7	6
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	78	0	123	83
Stage 1	-	-	-	-	76	-
Stage 2	-	-	-	-	47	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1521	-	872	976
Stage 1	-	-	-	-	947	-
Stage 2	-	-	-	-	975	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1482	-	845	945
Mov Cap-2 Maneuver	-	-	-	-	845	-
Stage 1	-	-	-	-	922	-
Stage 2	-	-	-	-	971	-
Approach	EB	WB	NB			
HCM Ctrl Dly, s/v	0	0.77	9.12			
HCM LOS				A		
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)	887	-	-	186	-	
HCM Lane V/C Ratio	0.014	-	-	0.003	-	
HCM Ctrl Dly (s/v)	9.1	-	-	7.4	0	
HCM Lane LOS	A	-	-	A	A	
HCM 95th %tile Q(veh)	0	-	-	0	-	

Scenario 6. Forecast Without PM Peak Street

Synchro 12 Report  
Page 5

# Exhibit E

## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

### *APPENDIX*

Level of Service Worksheets - Forecast 2028 With Project Conditions



# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Forecast With Project  
AM Peak School

Intersection						
Int Delay, s/veh	1.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘		↑			↑
Traffic Vol, veh/h	4	71	274	0	0	167
Future Vol, veh/h	4	71	274	0	0	167
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	69	69	69	69	69	36
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	6	103	397	0	0	464

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	861	397	0	-	-	-
Stage 1	397	-	-	-	-	-
Stage 2	464	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	326	652	-	0	0	-
Stage 1	679	-	-	0	0	-
Stage 2	633	-	-	0	0	-
Platoon blocked, %			-			-
Mov Cap-1 Maneuver	326	652	-	-	-	-
Mov Cap-2 Maneuver	326	-	-	-	-	-
Stage 1	679	-	-	-	-	-
Stage 2	633	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	12.05	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBTWBLn1	SBT
Capacity (veh/h)	- 619	-
HCM Lane V/C Ratio	- 0.176	-
HCM Ctrl Dly (s/v)	- 12	-
HCM Lane LOS	- B	-
HCM 95th %tile Q(veh)	- 0.6	-

# Exhibit E

## HCM 7th TWSC 2: Alameda Avenue & Parking Lot Access

Forecast With Project  
AM Peak School

Intersection						
Int Delay, s/veh	1.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	T		T			T
Traffic Vol, veh/h	30	21	286	38	25	137
Future Vol, veh/h	30	21	286	38	25	137
Conflicting Peds, #/hr	1	0	0	1	1	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	70	70	70	70	70	70
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	43	30	409	54	36	196

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	705	437	0	0	464	0
Stage 1	437	-	-	-	-	-
Stage 2	268	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	403	620	-	-	1097	-
Stage 1	651	-	-	-	-	-
Stage 2	777	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	387	619	-	-	1096	-
Mov Cap-2 Maneuver	387	-	-	-	-	-
Stage 1	651	-	-	-	-	-
Stage 2	748	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	14.34	0	1.3
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	458	278
HCM Lane V/C Ratio	-	-	0.159	0.033
HCM Ctrl Dly (s/v)	-	-	14.3	8.4
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.6	0.1

# Exhibit E

HCM 7th TWSC  
3: Alameda Avenue & Driveway/Elm Tree Lane

Forecast With Project  
AM Peak School

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	0	0	14	0	13	0	279	49	46	148	0
Future Vol, veh/h	0	0	0	14	0	13	0	279	49	46	148	0
Conflicting Peds, #/hr	0	0	2	2	0	0	2	0	1	1	0	2
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	76	76	76	76	76	76	76	76	76	76	76	76
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	0	0	18	0	17	0	367	64	61	195	0

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	685	750	199	718	718	400	197	0	0	433	0	0
Stage 1	318	318	-	400	400	-	-	-	-	-	-	-
Stage 2	367	433	-	318	318	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	362	340	842	344	355	650	1376	-	-	1127	-	-
Stage 1	694	654	-	626	601	-	-	-	-	-	-	-
Stage 2	652	582	-	694	654	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	331	318	839	322	332	649	1373	-	-	1126	-	-
Mov Cap-2 Maneuver	331	318	-	322	332	-	-	-	-	-	-	-
Stage 1	650	613	-	625	601	-	-	-	-	-	-	-
Stage 2	635	581	-	650	613	-	-	-	-	-	-	-

Approach	EB	WB	NB	SB
HCM Ctrl Dly, s/v	0	14.23	0	1.99
HCM LOS	A	B		

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1373	-	-	-	426	427	-
HCM Lane V/C Ratio	-	-	-	-	0.083	0.054	-
HCM Ctrl Dly (s/v)	0	-	-	0	14.2	8.4	0
HCM Lane LOS	A	-	-	A	B	A	A
HCM 95th %tile Q(veh)	0	-	-	-	0.3	0.2	-

# Exhibit E

## HCM 7th TWSC 4: Bus Access & Elm Tree Lane

Forecast With Project  
AM Peak School

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↑		↗
Traffic Vol, veh/h	95	4	0	27	0	4
Future Vol, veh/h	95	4	0	27	0	4
Conflicting Peds, #/hr	0	11	11	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	76	76	76	76	76	76
Heavy Vehicles, %	2	100	2	2	2	100
Mvmt Flow	125	5	0	36	0	5
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	-	-	-	139
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	4.2
Pot Cap-1 Maneuver	-	-	0	-	0	704
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	696
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB	WB		NB		
HCM Ctrl Dly, s/v	0	0		10.21		
HCM LOS				B		
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	696	-	-	-		
HCM Lane V/C Ratio	0.008	-	-	-		
HCM Ctrl Dly (s/v)	10.2	-	-	-		
HCM Lane LOS	B	-	-	-		
HCM 95th %tile Q(veh)	0	-	-	-		

# Exhibit E

## HCM Unsignalized Intersection Capacity Analysis 5: Annapolis Street & Elm Tree Lane

Forecast With Project  
AM Peak School



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↻			↻		
Traffic Volume (veh/h)	36	59	3	27	0	0
Future Volume (Veh/h)	36	59	3	27	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.39	0.39	0.39	0.39	0.39	0.39
Hourly flow rate (vph)	92	151	8	69	0	0
Pedestrians					63	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			306		316	231
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			306		316	231
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		100	100
cM capacity (veh/h)			1255		673	809
<b>Direction, Lane #</b>	<b>EB 1</b>	<b>WB 1</b>				
Volume Total	243	77				
Volume Left	0	8				
Volume Right	151	0				
cSH	1700	1255				
Volume to Capacity	0.14	0.00*				
Queue Length 95th (ft)	0	0				
Control Delay (s/veh)	0.0	0.9				
Lane LOS		A				
Approach Delay (s/veh)	0.0	0.9				
Approach LOS						
<b>Intersection Summary</b>						
Average Delay			0.2			
Intersection Capacity Utilization			16.1%	ICU Level of Service	A	
Analysis Period (min)			15			

\* Value less than 0.01.

# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Forecast With Project  
PM Peak School

Intersection						
Int Delay, s/veh	1.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘		↑			↑
Traffic Vol, veh/h	7	51	216	0	0	254
Future Vol, veh/h	7	51	216	0	0	254
Conflicting Peds, #/hr	1	2	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	9	64	270	0	0	318

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	589	272	0	-	-	-
Stage 1	270	-	-	-	-	-
Stage 2	319	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	471	767	-	0	0	-
Stage 1	775	-	-	0	0	-
Stage 2	737	-	-	0	0	-
Platoon blocked, %			-			-
Mov Cap-1 Maneuver	471	765	-	-	-	-
Mov Cap-2 Maneuver	471	-	-	-	-	-
Stage 1	775	-	-	-	-	-
Stage 2	736	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	10.63	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBTWBLn1	SBT
Capacity (veh/h)	- 711	-
HCM Lane V/C Ratio	- 0.102	-
HCM Ctrl Dly (s/v)	- 10.6	-
HCM Lane LOS	- B	-
HCM 95th %tile Q(veh)	- 0.3	-

# Exhibit E

## HCM 7th TWSC 2: Alameda Avenue & Parking Lot Access

Forecast With Project  
PM Peak School

Intersection						
Int Delay, s/veh	1.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	T		T			T
Traffic Vol, veh/h	34	18	251	16	25	231
Future Vol, veh/h	34	18	251	16	25	231
Conflicting Peds, #/hr	1	0	0	4	4	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	41	22	306	20	30	282

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	664	320	0	0	330	0
Stage 1	320	-	-	-	-	-
Stage 2	344	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	426	721	-	-	1230	-
Stage 1	736	-	-	-	-	-
Stage 2	718	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	411	718	-	-	1225	-
Mov Cap-2 Maneuver	411	-	-	-	-	-
Stage 1	733	-	-	-	-	-
Stage 2	696	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	13.58	0	0.78
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	483	176
HCM Lane V/C Ratio	-	-	0.131	0.025
HCM Ctrl Dly (s/v)	-	-	13.6	8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.4	0.1

# Exhibit E

## HCM 7th TWSC 3: Alameda Avenue & Driveway/Elm Tree Lane

Forecast With Project  
PM Peak School

Intersection												
Int Delay, s/veh	1.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	1	0	0	18	0	8	0	228	41	46	238	0
Future Vol, veh/h	1	0	0	18	0	8	0	228	41	46	238	0
Conflicting Peds, #/hr	0	0	0	0	0	0	6	0	4	4	0	6
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	0	21	0	9	0	265	48	53	277	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	655	707	283	677	683	293	283	0	0	317	0	0
Stage 1	390	390	-	293	293	-	-	-	-	-	-	-
Stage 2	265	317	-	384	390	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	379	360	756	367	372	746	1280	-	-	1243	-	-
Stage 1	634	608	-	715	670	-	-	-	-	-	-	-
Stage 2	740	654	-	639	608	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	354	339	752	347	349	743	1272	-	-	1239	-	-
Mov Cap-2 Maneuver	354	339	-	347	349	-	-	-	-	-	-	-
Stage 1	599	574	-	712	668	-	-	-	-	-	-	-
Stage 2	731	652	-	607	574	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Ctrl Dly, s/v	15.22		14.36		0		1.3	
HCM LOS	C		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1272	-	-	354	415	292	-	-
HCM Lane V/C Ratio	-	-	-	0.003	0.073	0.043	-	-
HCM Ctrl Dly (s/v)	0	-	-	15.2	14.4	8	0	-
HCM Lane LOS	A	-	-	C	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0	0.2	0.1	-	-

# Exhibit E

## HCM 7th TWSC 4: Bus Access & Elm Tree Lane

Forecast With Project  
PM Peak School

Intersection						
Int Delay, s/veh	0.3					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↑		↗
Traffic Vol, veh/h	88	4	0	26	0	4
Future Vol, veh/h	88	4	0	26	0	4
Conflicting Peds, #/hr	0	5	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	80	80	80	80	80	80
Heavy Vehicles, %	2	100	2	2	2	100
Mvmt Flow	110	5	0	33	0	5
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	-	-	-	118
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	7.2
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	4.2
Pot Cap-1 Maneuver	-	-	0	-	0	725
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	722
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB	WB	NB			
HCM Ctrl Dly, s/v	0	0	10.02			
HCM LOS				B		
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	722	-	-	-		
HCM Lane V/C Ratio	0.007	-	-	-		
HCM Ctrl Dly (s/v)	10	-	-	-		
HCM Lane LOS	B	-	-	-		
HCM 95th %tile Q(veh)	0	-	-	-		

# Exhibit E

## HCM Unsignalized Intersection Capacity Analysis 5: Annapolis Street & Elm Tree Lane

Forecast With Project  
PM Peak School



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔		↔			
Traffic Volume (veh/h)	38	54	4	26	0	0
Future Volume (Veh/h)	38	54	4	26	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.55	0.55	0.55	0.55	0.55	0.55
Hourly flow rate (vph)	69	98	7	47	0	0
Pedestrians					189	
Lane Width (ft)					0.0	
Walking Speed (ft/s)					3.5	
Percent Blockage					0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			356		368	307
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			356		368	307
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		100	100
cM capacity (veh/h)			1203		628	733
<b>Direction, Lane #</b>	<b>EB 1</b>	<b>WB 1</b>				
Volume Total	167	54				
Volume Left	0	7				
Volume Right	98	0				
cSH	1700	1203				
Volume to Capacity	0.10	0.00*				
Queue Length 95th (ft)	0	0				
Control Delay (s/veh)	0.0	1.1				
Lane LOS			A			
Approach Delay (s/veh)	0.0	1.1				
Approach LOS						
<b>Intersection Summary</b>						
Average Delay			0.3			
Intersection Capacity Utilization			16.7%	ICU Level of Service		A
Analysis Period (min)			15			

\* Value less than 0.01.

# Exhibit E

HCM 7th TWSC  
1: Annapolis St & Alameda Avenue

Forecast With Project  
PM Peak Street

Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘		↑			↑
Traffic Vol, veh/h	7	7	234	0	0	249
Future Vol, veh/h	7	7	234	0	0	249
Conflicting Peds, #/hr	2	3	0	5	2	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	7	7	249	0	0	265

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	516	252	0	-	-	-
Stage 1	249	-	-	-	-	-
Stage 2	267	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	-	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	-	-
Pot Cap-1 Maneuver	519	787	-	0	0	-
Stage 1	792	-	-	0	0	-
Stage 2	778	-	-	0	0	-
Platoon blocked, %			-			-
Mov Cap-1 Maneuver	518	784	-	-	-	-
Mov Cap-2 Maneuver	518	-	-	-	-	-
Stage 1	792	-	-	-	-	-
Stage 2	776	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	10.91	0	0
HCM LOS	B		

Minor Lane/Major Mvmt	NBTWBLn1	SBT
Capacity (veh/h)	- 624	-
HCM Lane V/C Ratio	- 0.024	-
HCM Ctrl Dly (s/v)	- 10.9	-
HCM Lane LOS	- B	-
HCM 95th %tile Q(veh)	- 0.1	-

# Exhibit E

## HCM 7th TWSC 2: Alameda Avenue & Parking Lot Access

Forecast With Project  
PM Peak Street

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔		↔			↔
Traffic Vol, veh/h	4	10	232	9	15	245
Future Vol, veh/h	4	10	232	9	15	245
Conflicting Peds, #/hr	0	1	0	5	5	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	82	82	82	82	82	82
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	5	12	283	11	18	299

Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	629	294	0	0	299	0
Stage 1	293	-	-	-	-	-
Stage 2	335	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	446	745	-	-	1262	-
Stage 1	757	-	-	-	-	-
Stage 2	724	-	-	-	-	-
Platoon blocked, %			-	-	-	-
Mov Cap-1 Maneuver	436	741	-	-	1256	-
Mov Cap-2 Maneuver	436	-	-	-	-	-
Stage 1	753	-	-	-	-	-
Stage 2	712	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	10.99	0	0.46
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	618	104
HCM Lane V/C Ratio	-	-	0.028	0.015
HCM Ctrl Dly (s/v)	-	-	11	7.9
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.1	0

# Exhibit E

## HCM 7th TWSC 3: Alameda Avenue & Driveway/Elm Tree Lane

Forecast With Project  
PM Peak Street

Intersection												
Int Delay, s/veh	0.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	1	0	0	18	0	3	0	209	34	23	242	0
Future Vol, veh/h	1	0	0	18	0	3	0	209	34	23	242	0
Conflicting Peds, #/hr	0	0	0	0	0	0	12	0	8	8	0	12
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	0	0	20	0	3	0	227	37	25	263	0

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	552	597	275	567	579	254	275	0	0	272	0	0
Stage 1	325	325	-	254	254	-	-	-	-	-	-	-
Stage 2	227	272	-	313	325	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	444	416	764	434	426	785	1288	-	-	1291	-	-
Stage 1	687	649	-	751	697	-	-	-	-	-	-	-
Stage 2	776	684	-	698	649	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	427	399	755	421	409	779	1273	-	-	1281	-	-
Mov Cap-2 Maneuver	427	399	-	421	409	-	-	-	-	-	-	-
Stage 1	664	627	-	745	692	-	-	-	-	-	-	-
Stage 2	772	679	-	682	627	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Ctrl Dly, s/v	13.45		13.41		0		0.68	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	1273	-	-	427	451	156	-	-
HCM Lane V/C Ratio	-	-	-	0.003	0.051	0.02	-	-
HCM Ctrl Dly (s/v)	0	-	-	13.4	13.4	7.9	0	-
HCM Lane LOS	A	-	-	B	B	A	A	-
HCM 95th %tile Q(veh)	0	-	-	0	0.2	0.1	-	-

# Exhibit E

## HCM 7th TWSC 4: Bus Access & Elm Tree Lane

Forecast With Project  
PM Peak Street

Intersection						
Int Delay, s/veh	0					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↑		↗
Traffic Vol, veh/h	57	0	0	21	0	0
Future Vol, veh/h	57	0	0	21	0	0
Conflicting Peds, #/hr	0	7	7	0	7	1
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	84	84	84	84	84	84
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	68	0	0	25	0	0
Major/Minor	Major1	Major2	Minor1			
Conflicting Flow All	0	0	-	-	-	76
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	-	-	-	-	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	-	-	-	-	3.318
Pot Cap-1 Maneuver	-	-	0	-	0	985
Stage 1	-	-	0	-	0	-
Stage 2	-	-	0	-	0	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	-	-	-	978
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB	WB	NB			
HCM Ctrl Dly, s/v	0	0	0			
HCM LOS						A
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBT		
Capacity (veh/h)	-	-	-	-		
HCM Lane V/C Ratio	-	-	-	-		
HCM Ctrl Dly (s/v)	0	-	-	-		
HCM Lane LOS	A	-	-	-		
HCM 95th %tile Q(veh)	-	-	-	-		

# Exhibit E

## HCM Unsignalized Intersection Capacity Analysis 5: Annapolis Street & Elm Tree Lane

Forecast With Project  
PM Peak Street



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔		↔			
Traffic Volume (veh/h)	31	26	9	21	0	0
Future Volume (Veh/h)	31	26	9	21	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.71	0.71	0.71	0.71	0.71	0.71
Hourly flow rate (vph)	44	37	13	30	0	0
Pedestrians	2			7	27	
Lane Width (ft)	12.0			12.0	0.0	
Walking Speed (ft/s)	3.5			3.5	3.5	
Percent Blockage	0			1	0	
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			108		148	97
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			108		148	97
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		100	100
cM capacity (veh/h)			1483		836	953
<b>Direction, Lane #</b>	<b>EB 1</b>	<b>WB 1</b>				
Volume Total	81	43				
Volume Left	0	13				
Volume Right	37	0				
cSH	1700	1483				
Volume to Capacity	0.05	0.00*				
Queue Length 95th (ft)	0	1				
Control Delay (s/veh)	0.0	2.3				
Lane LOS			A			
Approach Delay (s/veh)	0.0	2.3				
Approach LOS						
<b>Intersection Summary</b>						
Average Delay			0.8			
Intersection Capacity Utilization			21.8%	ICU Level of Service	A	
Analysis Period (min)			15			

\* Value less than 0.01.

# Exhibit E

## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

*APPENDIX*  
ITE Parking Generation Worksheets



# Exhibit E

## Elementary School (520)

**Peak Period Parking Demand vs: Students**

On a: Weekday (Monday - Friday)

Setting/Location: General Urban/Suburban

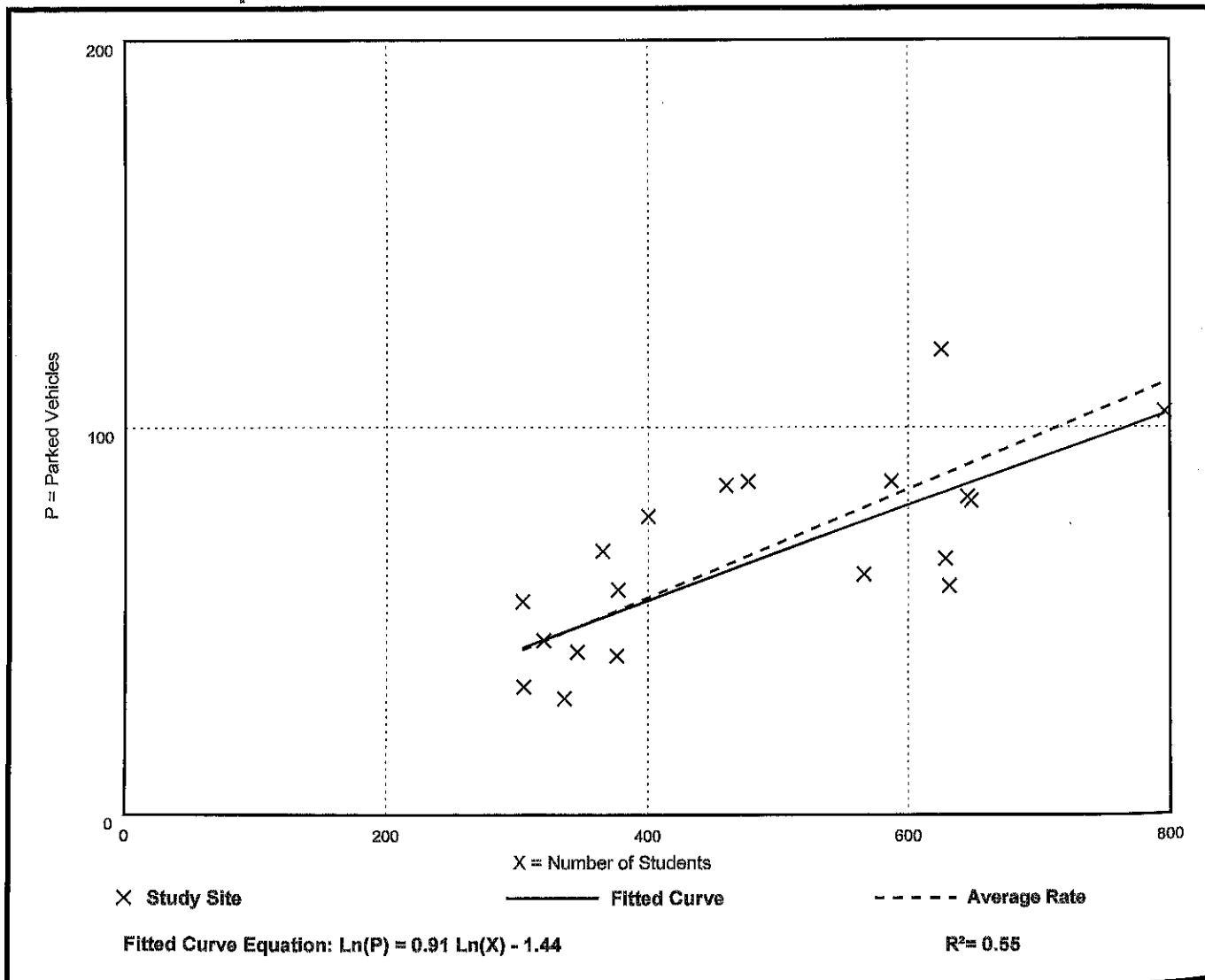
Number of Studies: 19

Avg. Num. of Students: 484

### Peak Period Parking Demand per Student

Average Rate	Range of Rates	33rd / 85th Percentile	95% Confidence Interval	Standard Deviation (Coeff. of Variation)
0.14	0.09 - 0.19	0.12 / 0.19	***	0.03 ( 21% )

### Data Plot and Equation



# Exhibit E

## WHITTIER ELEMENTARY SCHOOL TRAFFIC IMPACT ANALYSIS

*APPENDIX*  
Site Plan



